

International Workshop on Advances in Assessment and Modeling of Earthquake Loss



International Workshop on Advances in Assessment and Modeling of Earthquake Loss, which is organized by the TCIP, will bring the insurance industry players, international reinsurance and modeling companies, government agencies and universities on November 4–5, 2019 in Istanbul, Hotel Wyndham Grand Levent.

EMPIRICAL FRAGILITY AND VULNERABILITY OF REGIONAL BUILDING STOCKS IN EUROPE

Sergio Lagomarsino
sergio.lagomarsino@unige.it



**Università
di Genova**

**DICCA – Department of Civil,
Chemical and Environmental
Engineering**

OUTLINE OF THE PRESENTATION

FRAGILITY CURVES

What do they represent?

Vulnerability as a component of seismic risk and loss assessment

What do they depend on?

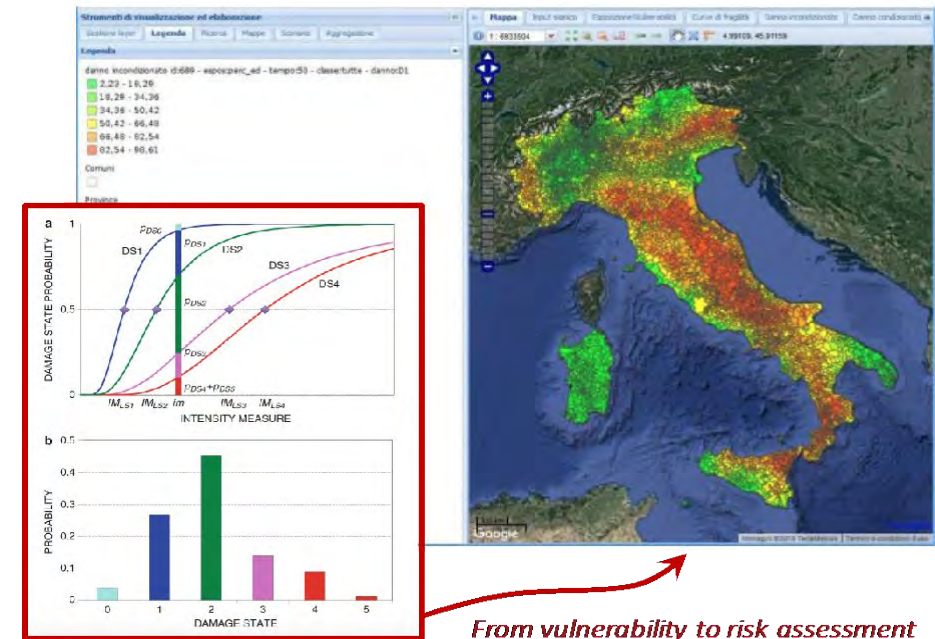
Involved dispersions & influence on results of seismic risk analysis

How are they obtained?

Overview of methods & focus on macroseismic and empirical ones

How can they be used?

Practical issues & application to the Italian seismic risk assessment



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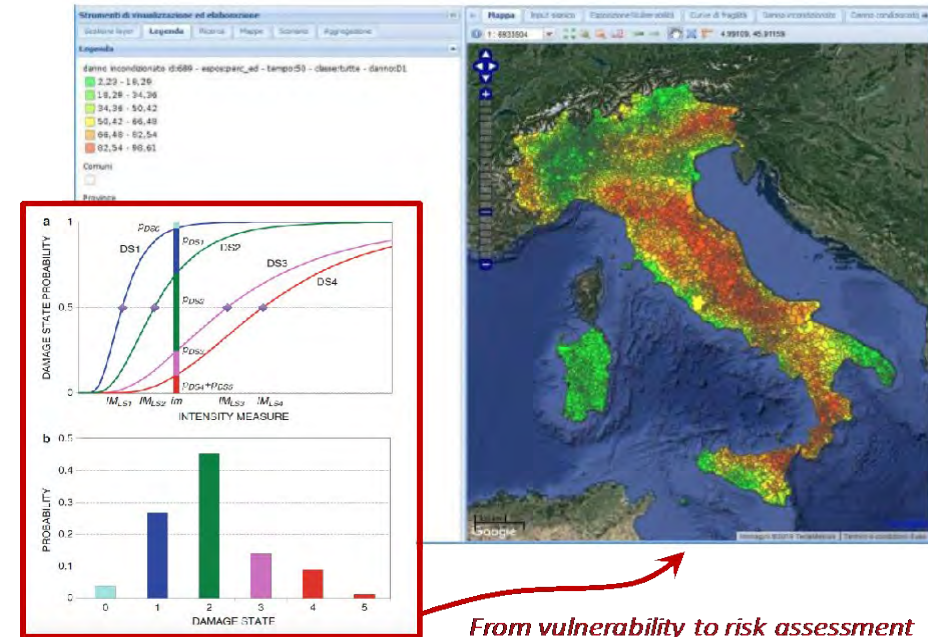
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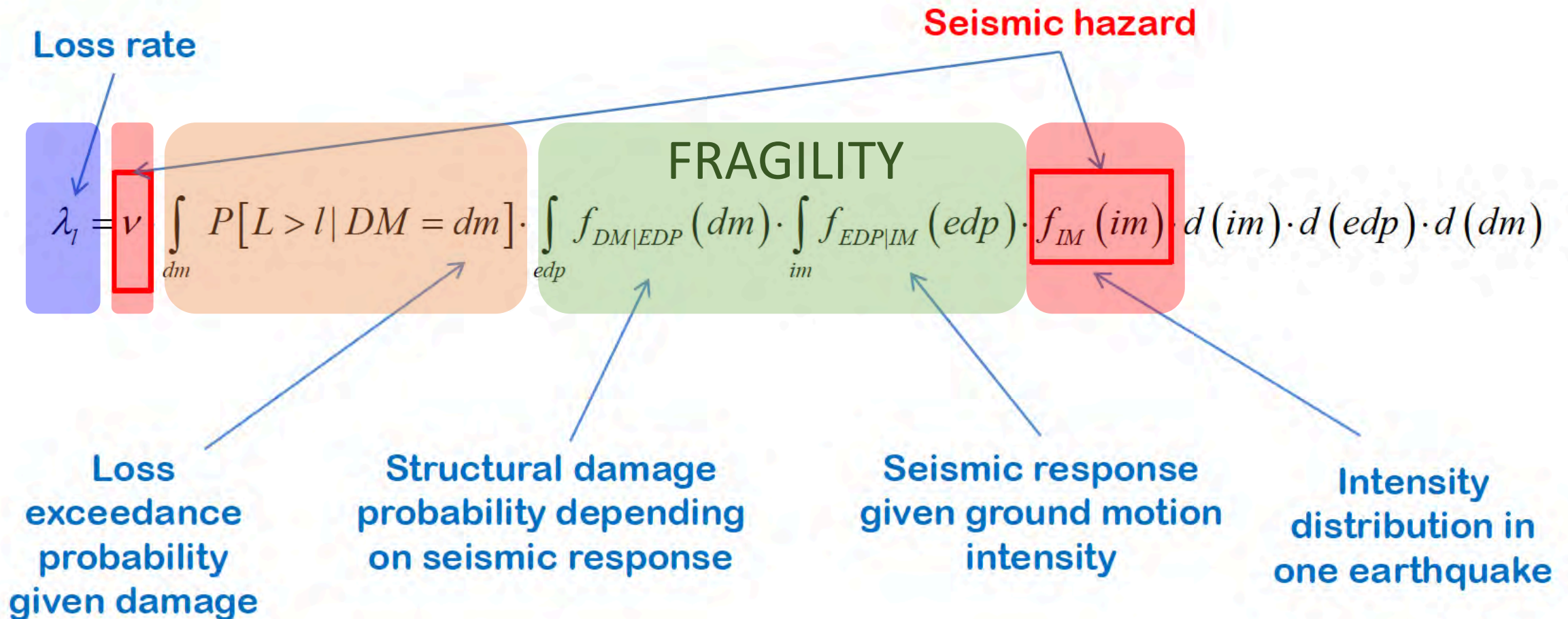
RISK ANALYSIS



The risk analysis at territorial scale is **intrinsically probabilistic** (as the PEER-PBA at scale of the single building): it is the result of **convolution of various sources of uncertainties and dispersions**

RISK ANALYSIS – BASICS

It represents the **EXPECTED RATE** in a **GIVEN TIME** (e.g. 1 year, 50 years) of possible **LOSSES** (economical, buildings usability, casualties, ...) due to the **DAMAGE** occurred in the building stock and considering people (**EXPOSURE**) in a **GIVEN AREA** (e.g. the municipality, the region, the whole country) as a consequence of possible seismic events (**HAZARD**)



RISK ANALYSIS – HAZARD

Hazard curve (from PSHA - Probabilistic Seismic Hazard Assessment):
annual probability of exceedance of a selected intensity measure of the earthquake at the given site

UNCONDITIONED

TIME-BASED

Evaluates the loss **in a specified period of time** (e.g. 1 year, 30 or 50 years) considering **all earthquakes** that could occur in that time period, each one with the specific probability of occurrence

The **time period** depends on the aims of the decision-makers.
Assessments based on **one year** are useful **for cost-benefit analysis**

CONDITIONED

INTENSITY-BASED

Evaluates the loss assuming the **earthquake intensity** associated to a given return period. In general the design earthquakes consistent with Building Codes are adopted.

Useful for **comparative analyses** by considering RARE and FREQUENT events.

CONDITIONED

SCENARIO-BASED

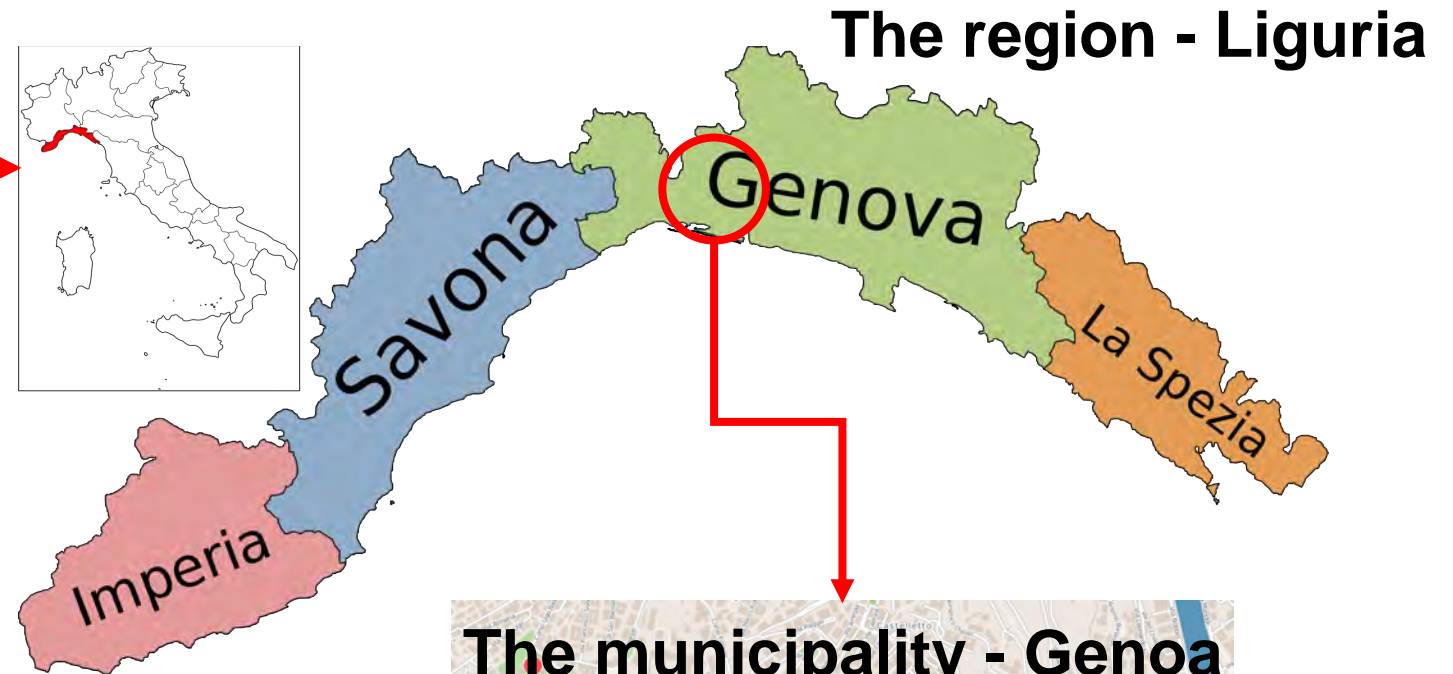
Evaluates the loss assuming a **shaking scenario**, relative to the area under examination, derived by a **deterministic earthquake**, in terms of **magnitude, epicenter location, etc.**

Useful to support the design of **seismic emergency plan** at municipality scale.

In case of CONDITIONED ASSESSMENT IT IS POSSIBLE ALSO
ADOPT THE MACROSEISMIC INTENSITY as INTENSITY MEASURE

according to the notation introduced by FEMA P-58 for performance assessment of buildings

RISK ANALYSIS – *different scales*



RISK ANALYSIS – EXPOSURE

Risk analyses at large scale can be referred to:

- ❑ **building stocks** characterized by data **aggregated in sub-areas** – *typical of the assessment on residential buildings that are present in the area under investigation*
- ❑ **buildings portfolio** characterized by a **group of single structures** – *typical of the assessment on strategic buildings in the area (e.g. schools, strategic buildings, ...)*

The analysis of the exposure is functional to define the vulnerability, therefore:

- ❑ **Taxonomy**: aimed to define the attributes that influence the vulnerability
- ❑ **Classification**: grouping of buildings for which it is assumed the same behavior (using the taxonomy tags, you may end up with a huge number of classes, and it is not straightforward that their behavior is really different)
- ❑ **Inventory**: taxonomy and classification must take into account the available information

FRAGILITY CURVES: TAXONOMY & CLASSIFICATION

GEM Building Taxonomy Version 2.0

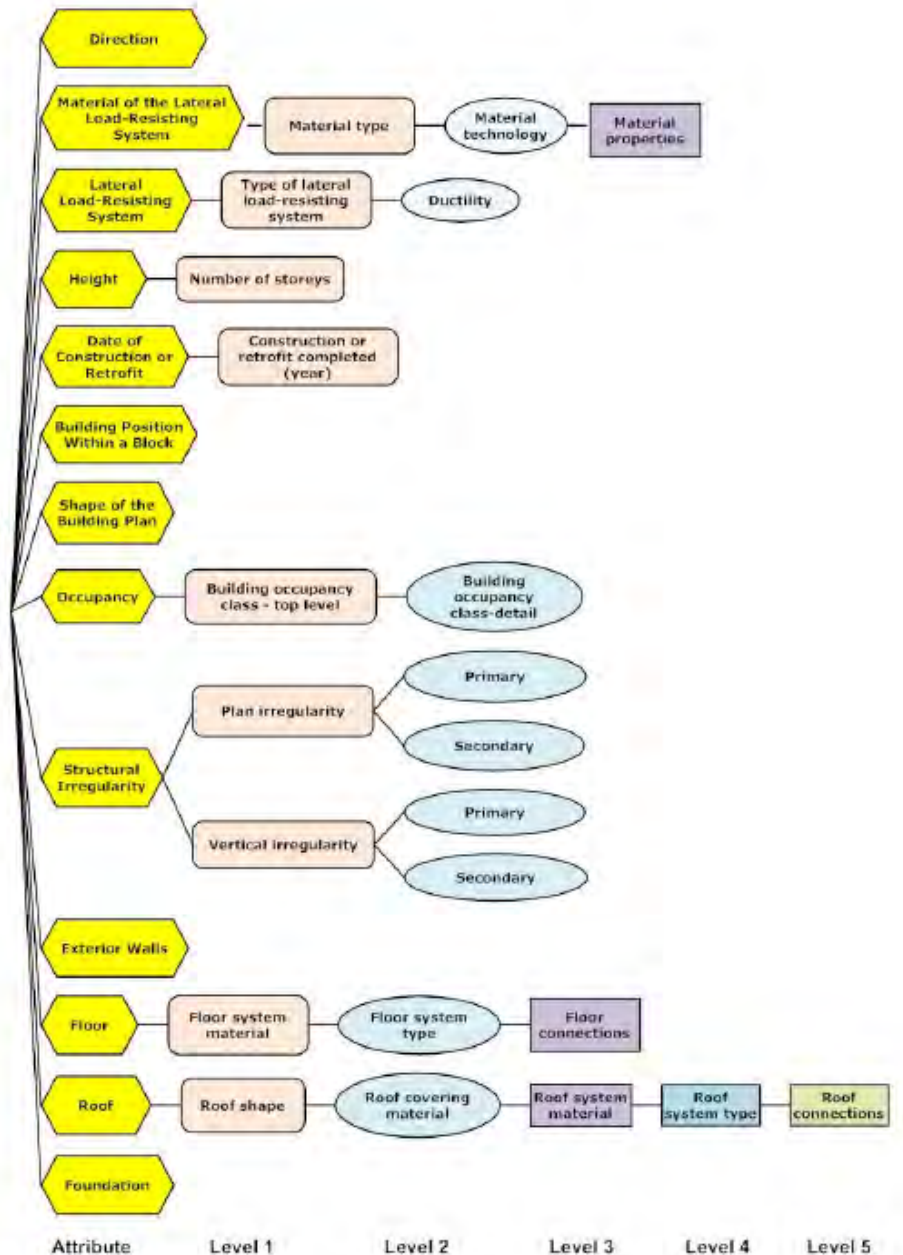
GEM Technical Report 2013-02

Version: 1.0.0

Date: November 2013

LIST OF 13 ATTRIBUTES

1. Direction
2. Material of the lateral load-resisting system
3. Lateral load-resisting system
4. Height
5. Date of construction or retrofit
6. Occupancy
7. Building position within a block
8. Shape of the building plan
9. Structural irregularity
10. Exterior walls
11. Roof
12. Floor
13. Foundation system



Building classes (combination of tags) may become too many and should be completed in describing the stock.

CLASSIFICATION OF RESIDENTIAL BUILDING STOCK IN ITALY FROM CENSUS DATA

ISTAT - Census of the population

Information available at municipality level, disaggregated in terms of:

Exposure:

- Population
- # of buildings, # flats and size

Vulnerability:

- Type (material):
 - Masonry, Reinforced concrete, other
- Building height:
 - Low 1-3, Medium 3-5, High >5
- Age of construction:
 - < 1919, 1919-1945, 1946-1961, 1962-1981, 1982-1991, >1991



EUCENTRE
FOR YOUR SAFETY.

Italian seismic risk maps by IRMA Platform



DPC/ReLUIs Project 2019-2021

- **CARTIS – Inventory of building typologies**
Coordinator: Giulio Zuccaro
- **MARS – Seismic Risk Maps and Damage Scenario**
Coordinators: Sergio Lagomarsino and Angelo Masi



Regionalization of fragility curves:

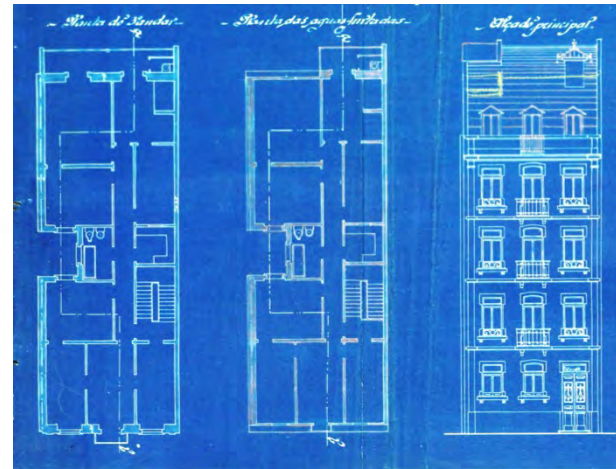
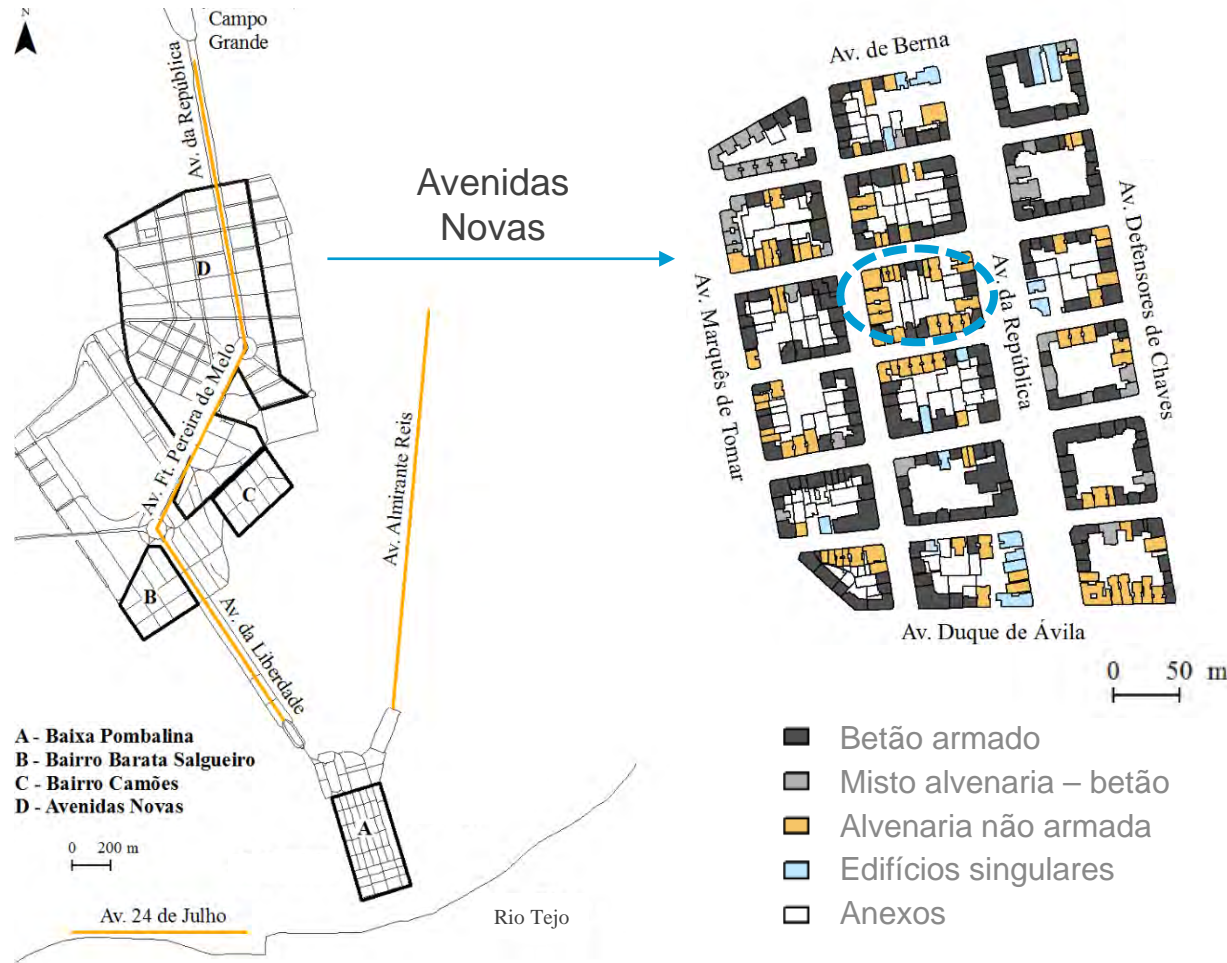
- Identification of sub-regions in which buildings have similar characteristics, by means of sample surveys on small areas, but statistically representative.

Masonry buildings, age 1919-1945, height 3 to 5 stories

Region	Masonry		Horizontal floors			
	Brick masonry	Stone masonry	Vaults	Timber	Steel	R.C.
Emilia Romagna	95%	5%	15%	40%	40%	5%
Abruzzo	15%	85%	20%	20%	35%	25%

CLASSIFICATION OF THE BUILDING STOCK AT URBAN LEVEL

The case study of GAIOLEIRO buildings in Lisbon between XIX and XX centuries



CLASSIFICATION OF THE BUILDING STOCK AT URBAN LEVEL

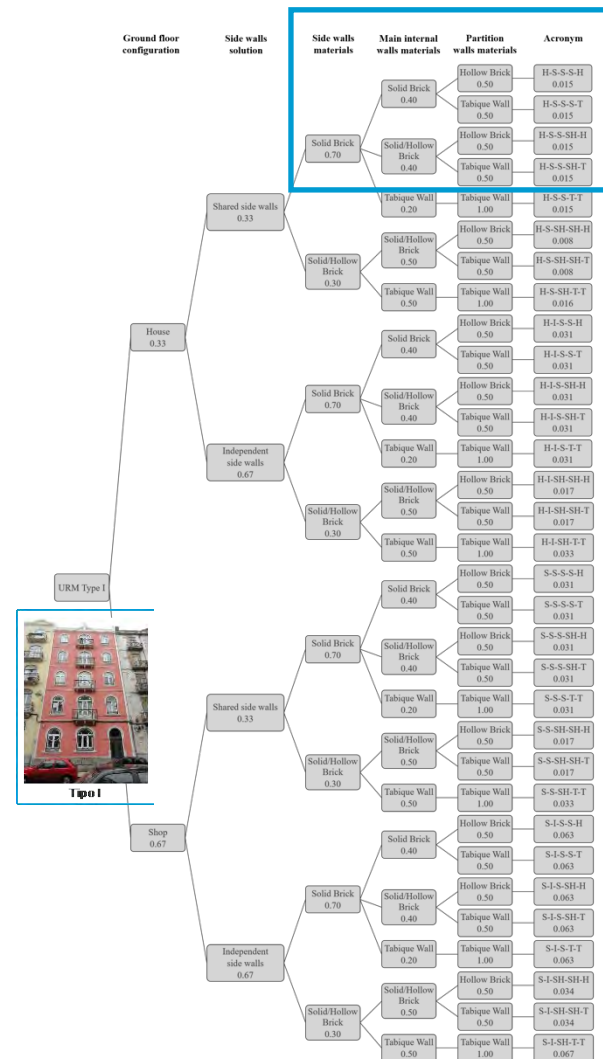
The case study of GAIOLEIRO buildings in Lisbon between XIX and XX centuries



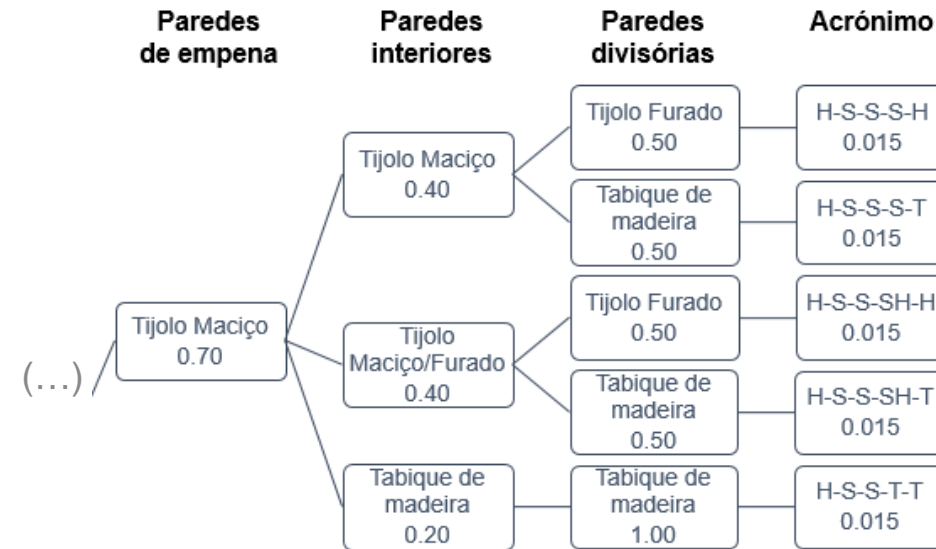
REF: Simoes et al. (2019) Fragility functions for tall URM buildings around early 20° century in Lisbon. Part 1 & 2. International Journal of Architectural Heritage, online.

CLASSIFICATION OF THE BUILDING STOCK AT URBAN LEVEL

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Epistemic Uncertainties: geometry, structural details and materials

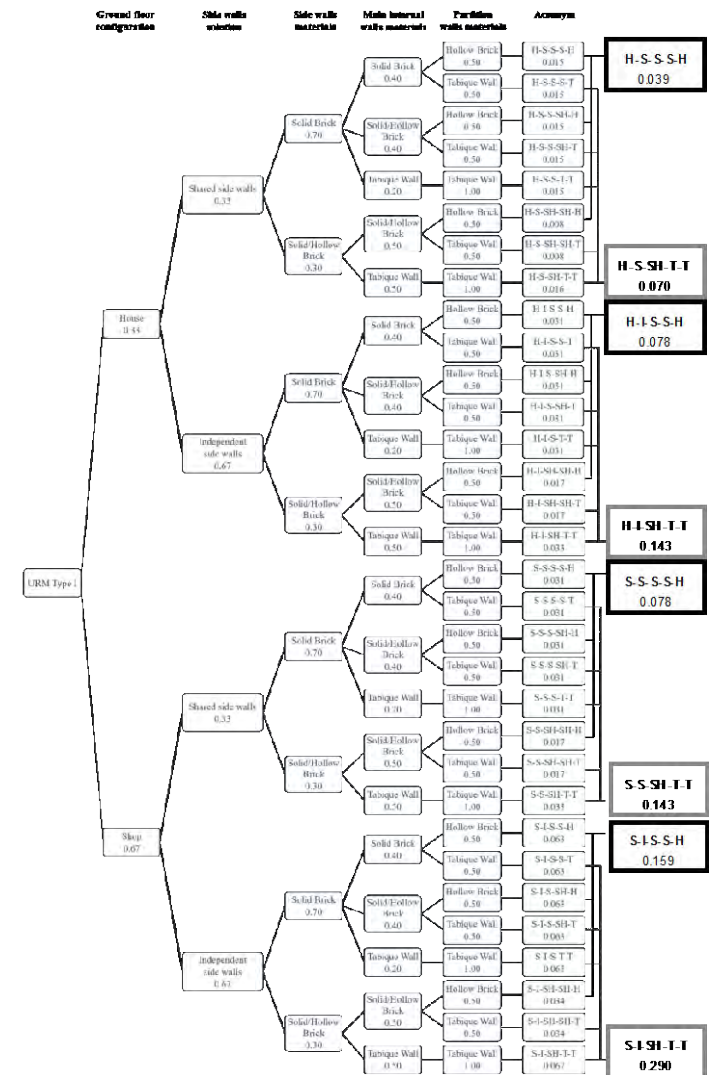


- Configuração R/C: habitação ou comércio
- Solução paredes de empena: meeiras ou independentes
- Paredes de empena: tijolo maciço ou tijolo furado
- Paredes interiores: tijolo maciço ou tijolo furado
- Paredes divisórias: tijolo furado ou tabique de madeira

LOGIC TREE WITH 32 BRANCHES

The case study of GAIOLEIRO buildings in Lisbon between XIX and XX centuries

➔ from 32 to 8 branches



REF: Simoes et al. (2019) Fragility functions for tall URM buildings around early 20^o century in Lisbon. Part 1 & 2. International Journal of Architectural Heritage, online.

RISK ANALYSIS – DAMAGE METRIC

$$\lambda_l = \nu \int_{dm} P[L > l | DM = dm] \cdot \int_{edp} f_{DM|EDP}(dm) \cdot \int_{im} f_{EDP|IM}(edp) \cdot \underbrace{f_{IM}(im)}_{\text{Intensity Measure}} \cdot d(im) \cdot d(edp) \cdot d(dm)$$

DAMAGE LEVEL ACCORDING TO EMS 98 (Grunthal 1998)



GRADE 1:
Negligible to slight damage



GRADE 2:
Moderate damage



GRADE 3:
Substantial to heavy damage

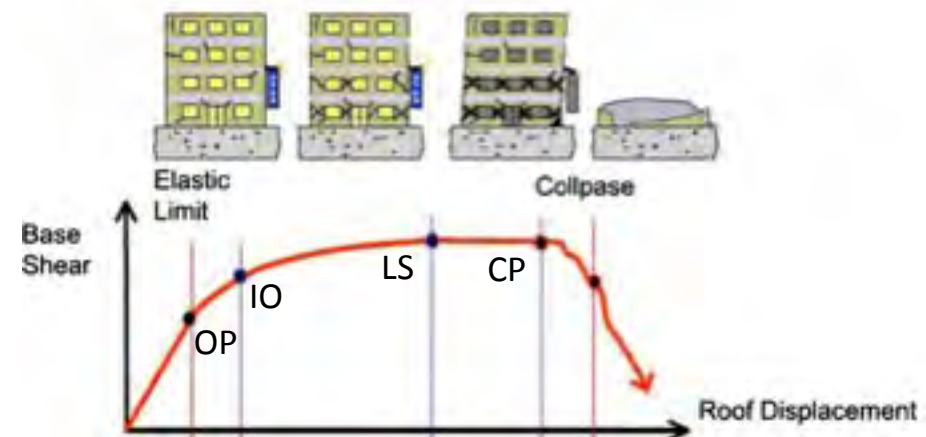


GRADE 4:
Very heavy damage



GRADE 5:
Destruction

- Usually, the damage is described in **DISCRETE** terms rather than as a CONTINUOUS variable (→ integral becomes a sum)
- The **5 grades** adopted by the EMS 98 scale may be used (the first four are similar to the LSs of seismic codes)
- DAMAGE LEVELS** are correlated to the EDPs (Engineering Demand Parameters), representative of structural response, and are usually related to PERFORMANCE LEVELS

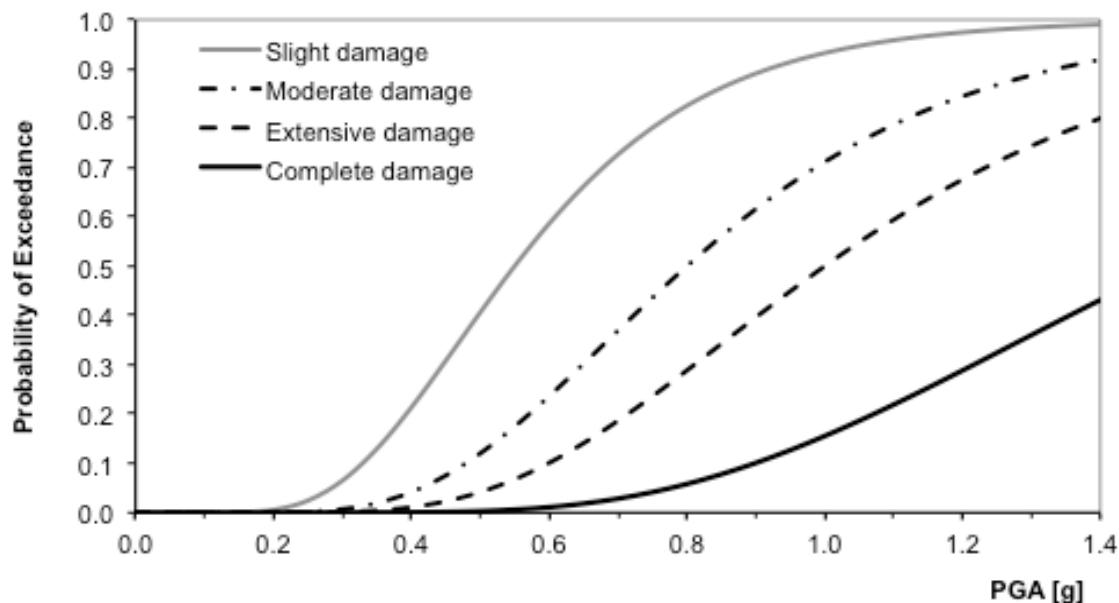


RISK ANALYSIS – FRAGILITY

$$\lambda_l = \nu \int_{dm} P[L > l | DM = dm] \cdot \int_{edp} f_{DM|EDP}(dm) \cdot \int_{im} f_{EDP|IM}(edp) \cdot f_{IM}(im) \cdot d(im) \cdot d(edp) \cdot d(dm)$$

If the damage is described by a CONTINUOUS variable $\int_{im} f_{DM|IM}(dm) f_{IM}(im) d(im)$

DISCRETE damage variable $f_{DM|IM}(dm) = P[dm \geq DM | im] = P[IM_{DM} < im] = \Phi \left[\frac{\log \left(\frac{im}{IM_{DM}} \right)}{\beta_{DM}} \right]$



lognormal fragility curves

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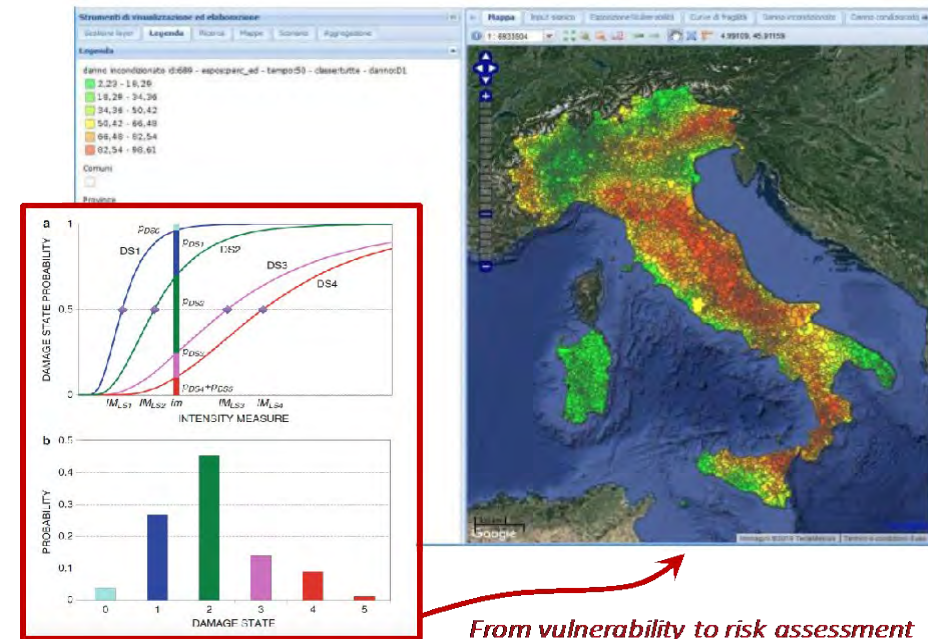
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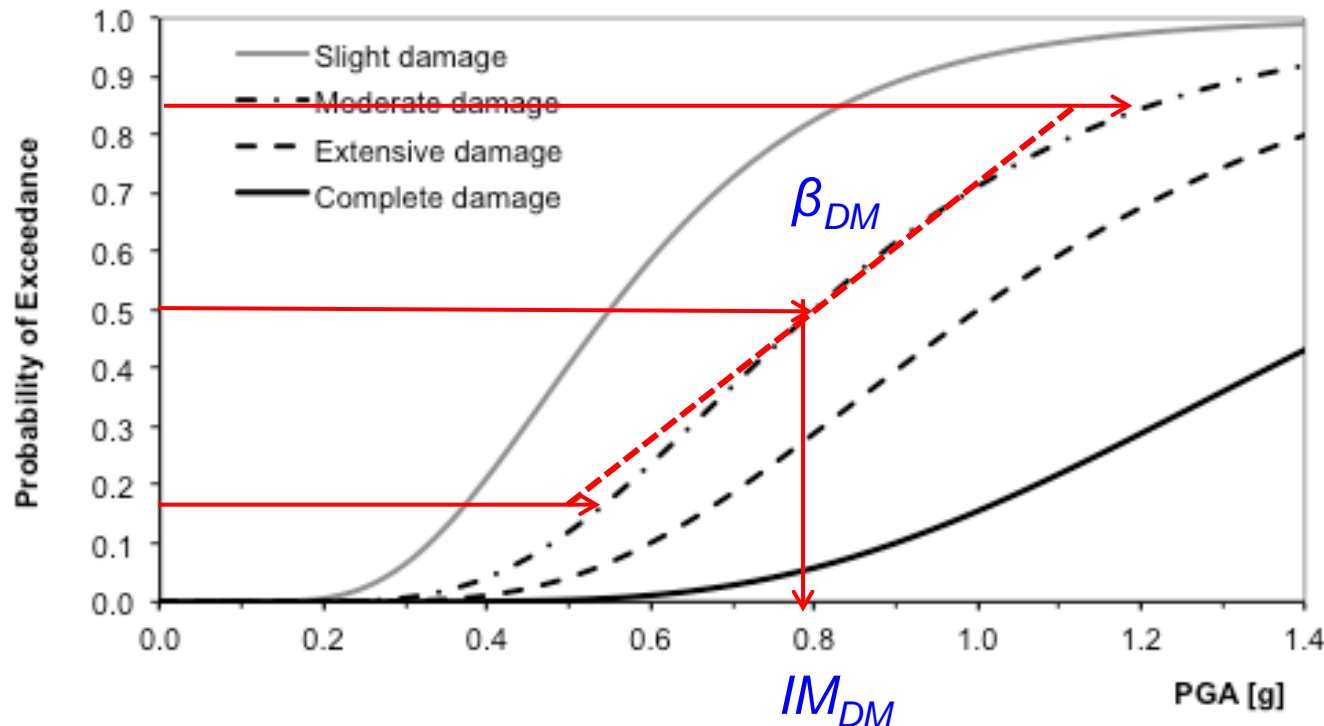


FRAGILITY FUNCTIONS

The fragility function of a building class gives the probability that a Damage Levels (DM) is reached given a value im of the Intensity Measure (IM) :

$$P[dm \geq DM|im] = P[IM_{DM} < im] = \Phi \left[\frac{\log \left(\frac{im}{IM_{DM}} \right)}{\beta_{DM}} \right]$$

where: IM_{DM} is the median value of the lognormal distribution of the intensity measure for which the DM is attained and β_{DM} is the dispersion.



For IM different possible choice
the more IM is effective and the less is β

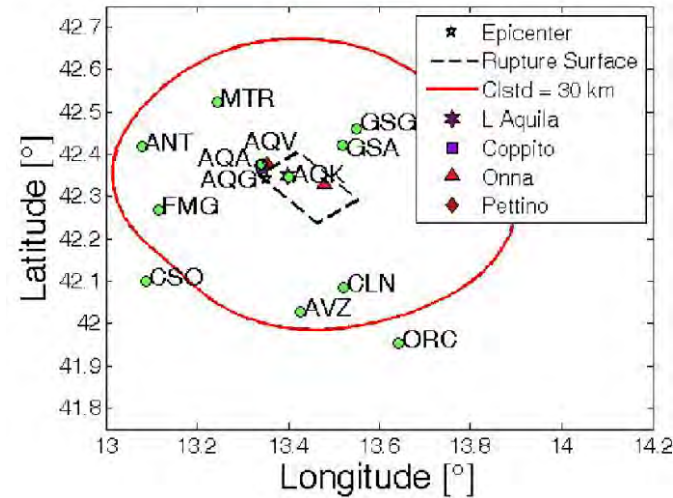
$$\beta_{DM} = \frac{1}{2} [\log(IM_{84}) - \log(IM_{16})]$$

*the more the building class is
homogenous and the less is β*

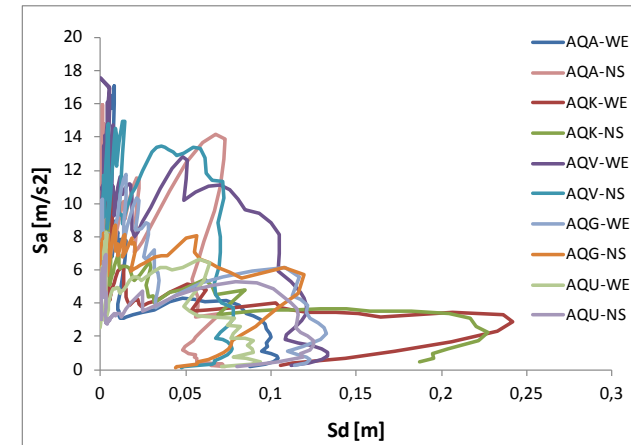
FRAGILITY FUNCTIONS – *involved uncertainties*

$$\beta_{DM} = \sqrt{\beta_{records}^2 + \beta_{capacity}^2 + \beta_{damage\ level}^2}$$

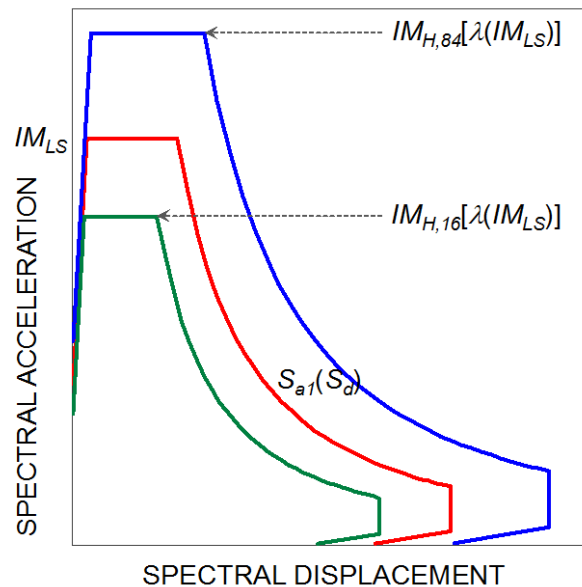
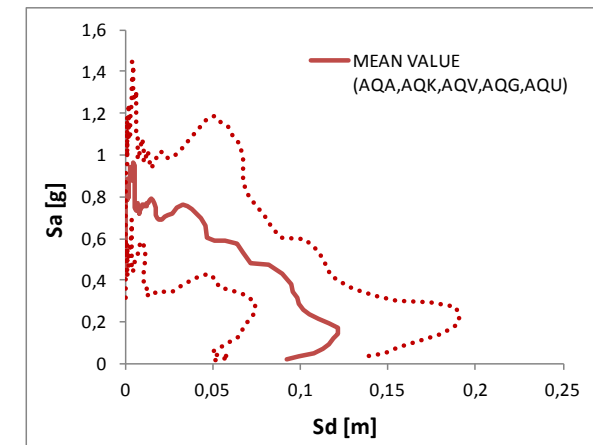
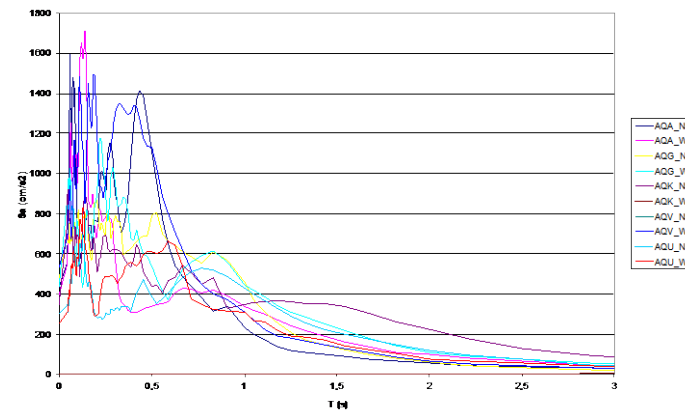
L'Aquila 2009 event – recordings from the 10 available stations on the area



ADRS acceleration spectrum format



Sa- T acceleration spectrum format

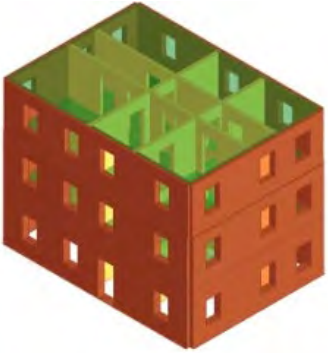


FRAGILITY FUNCTIONS – *involved uncertainties*

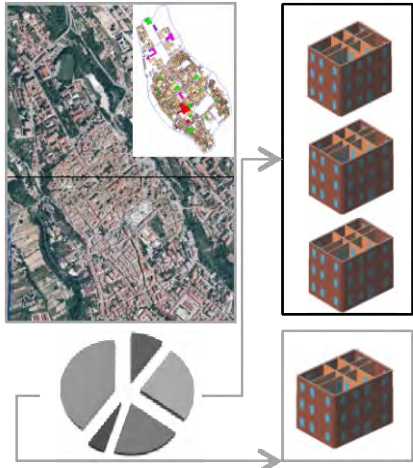
$$\beta_{DM} = \sqrt{\beta_{records}^2 + \beta_{capacity}^2 + \beta_{damage\ level}^2}$$

SINGLE BUILDING

Single structure at a given site

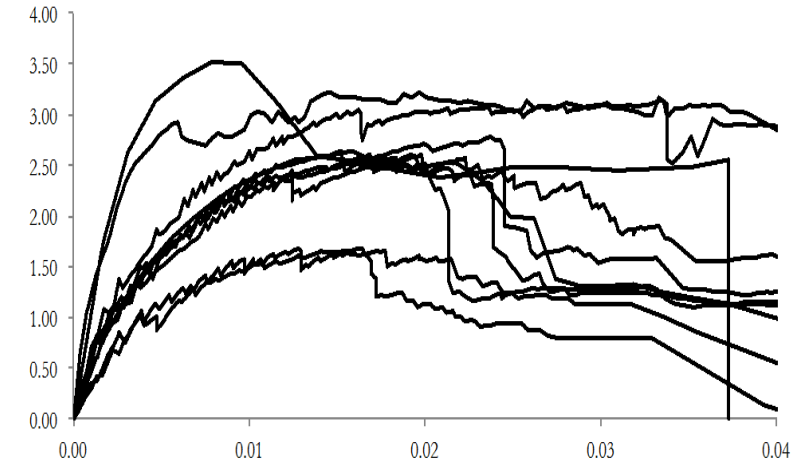
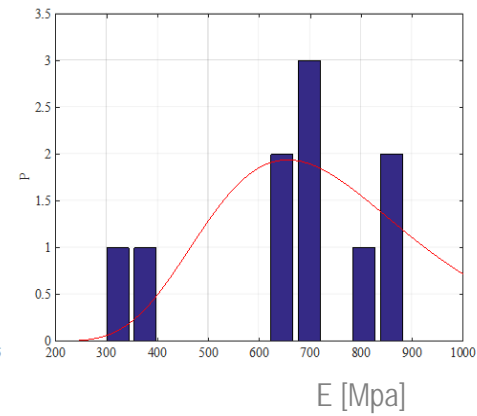
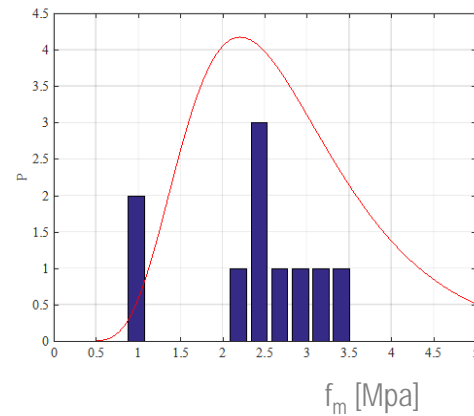


Classes of buildings with a similar seismic behavior



REGIONAL SCALE

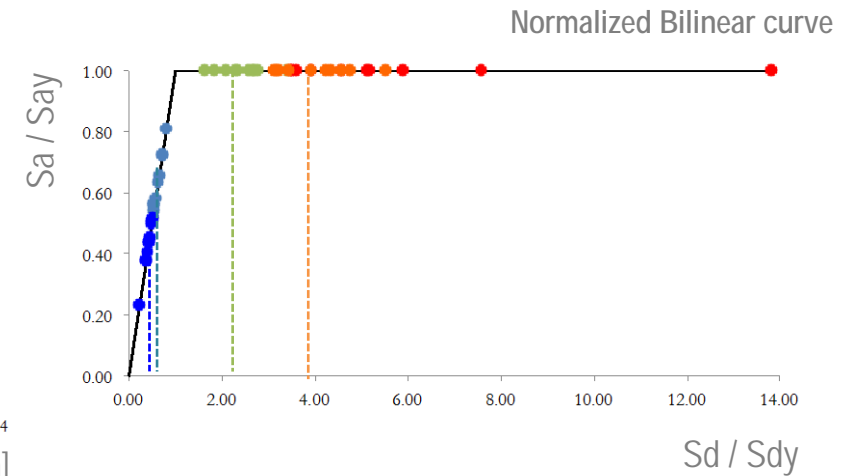
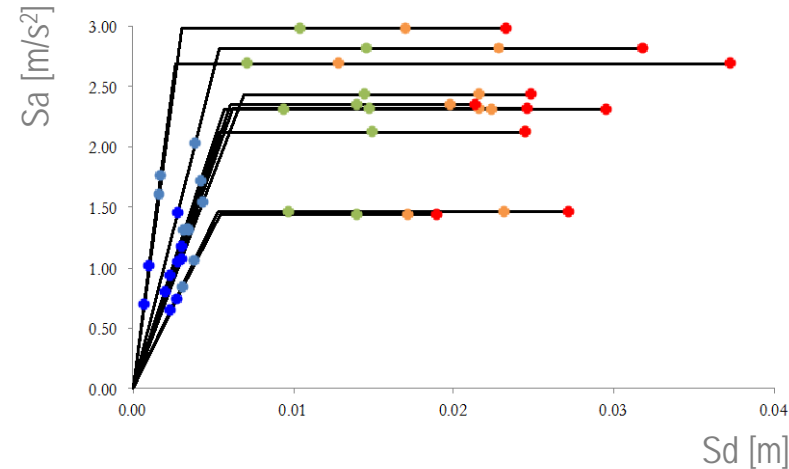
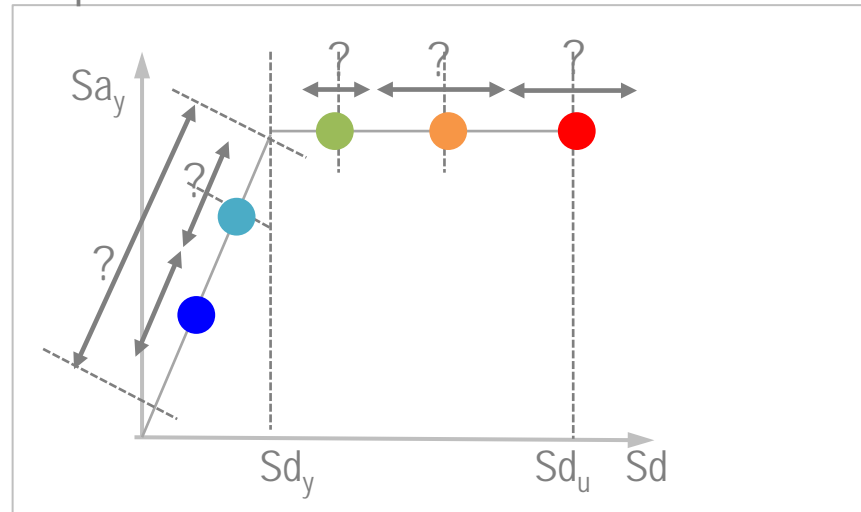
Variability of parameters (mechanical, geometric, ...) characteristic of the **WHOLE CLASS OF BUILDINGS**



FRAGILITY FUNCTIONS – *involved uncertainties*

$$\beta_{DM} = \sqrt{\beta_{records}^2 + \beta_{capacity}^2 + \beta_{damage\ level}^2}$$

Simplified Bilinear Curve

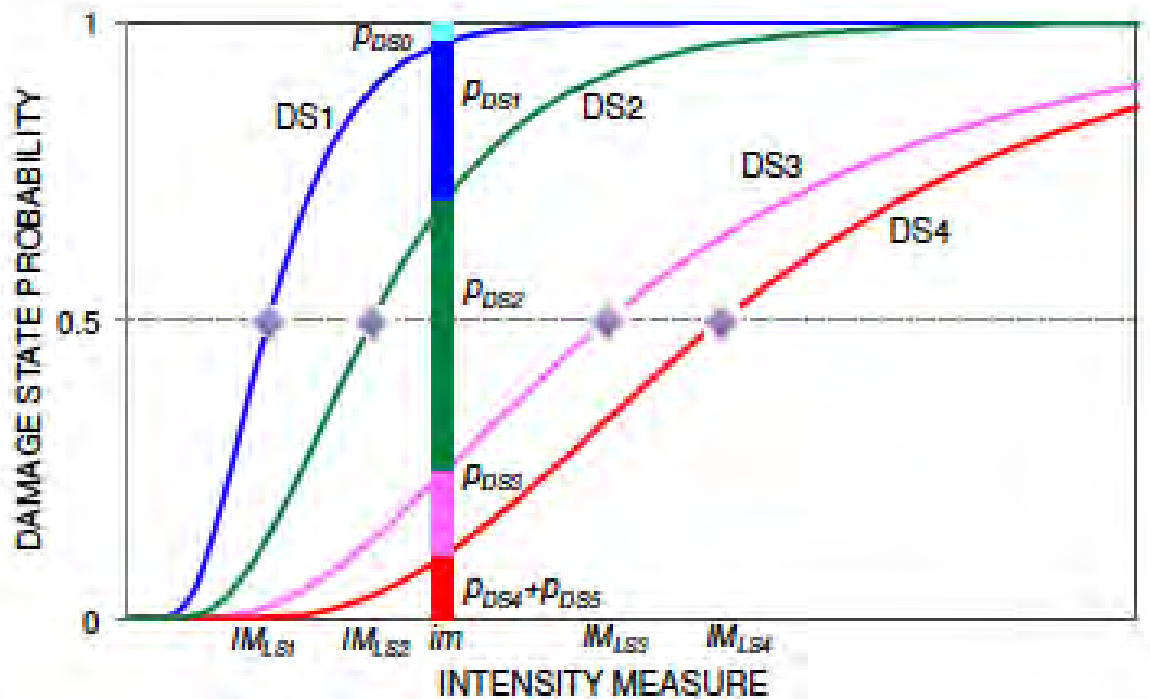


■ negligible ■ DL1 ■ DL2 ■ DL3 ■ DL4

FRAGILITY FUNCTIONS – DAMAGE PROBABILITY MATRIX (DPM)

FRAGILITY OF A BUILDING STOCK

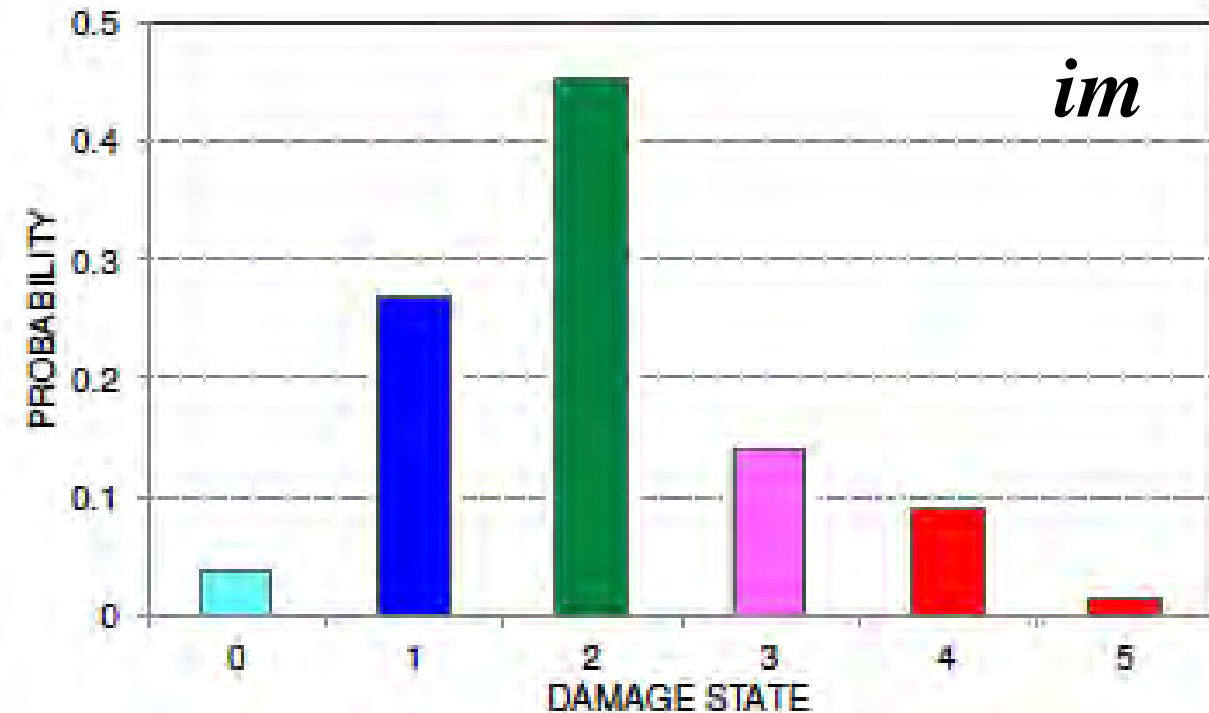
is represented by a set of curves that may be defined by the IM_{Dk} , the dispersion β and the distance between damage levels (ductility)



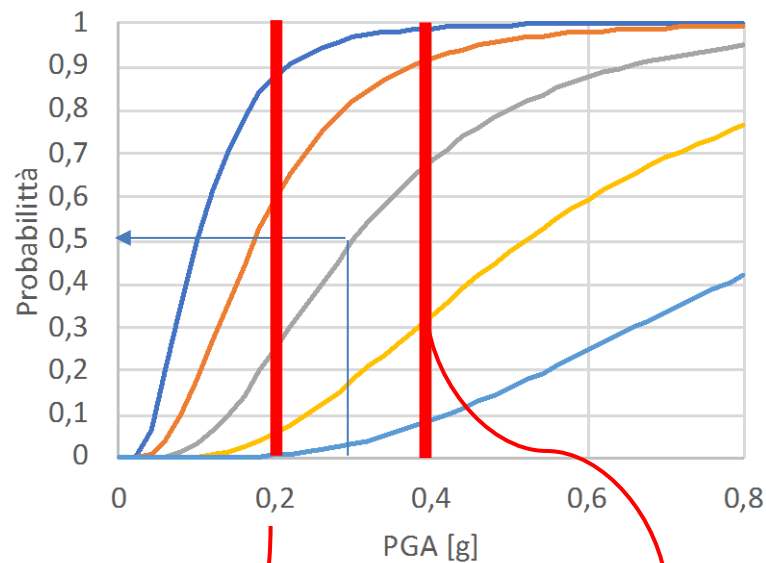
im

DAMAGE LEVEL HISTOGRAM

(when a discrete IM is used, like the macroseismic intensity, the vulnerability is defined by a DPM – Damage Probability Matrix)



FRAGILITY FUNCTIONS – influence of β

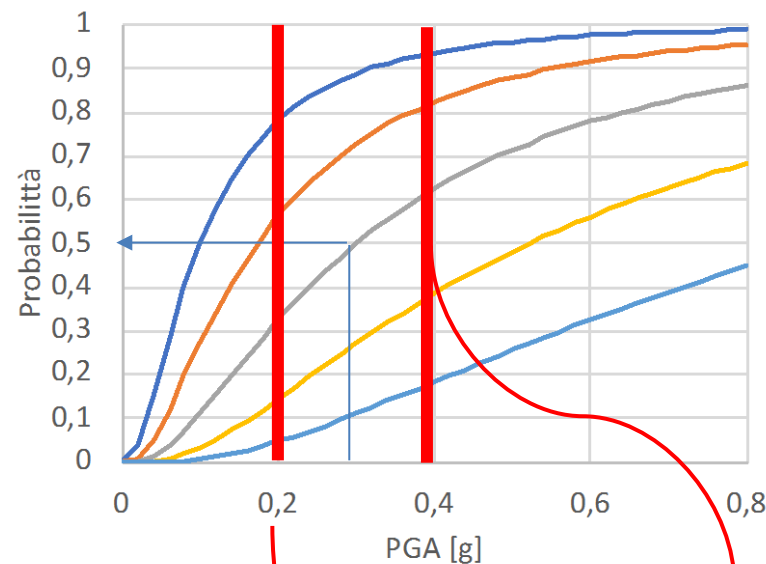
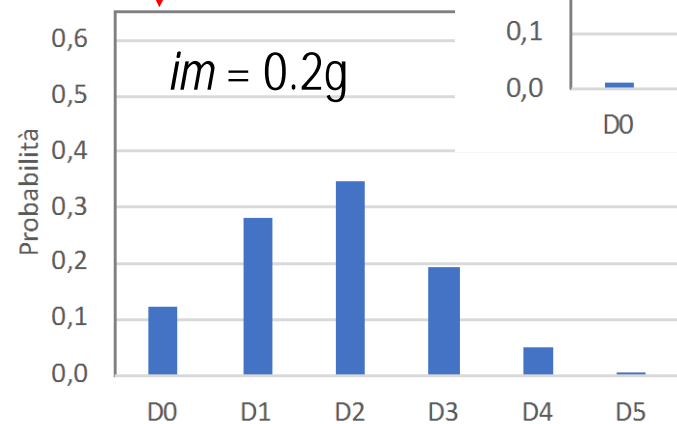
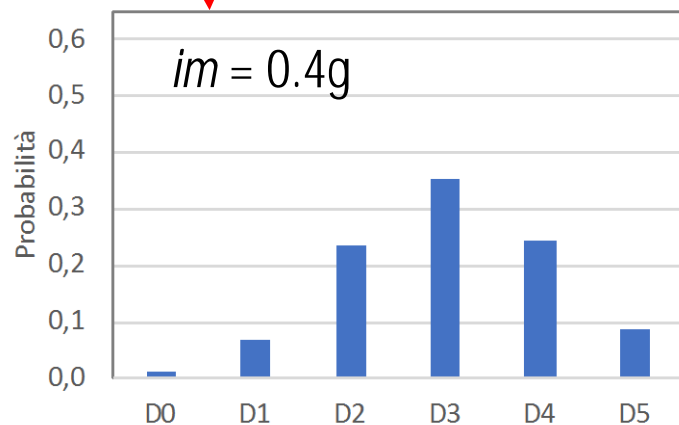


$$IM_{D3} = 0.3g$$

$$\beta_{D3} = 0.6$$

$$IM_{D1} = 0.1g$$

$$IM_{D5} = 0.9g$$

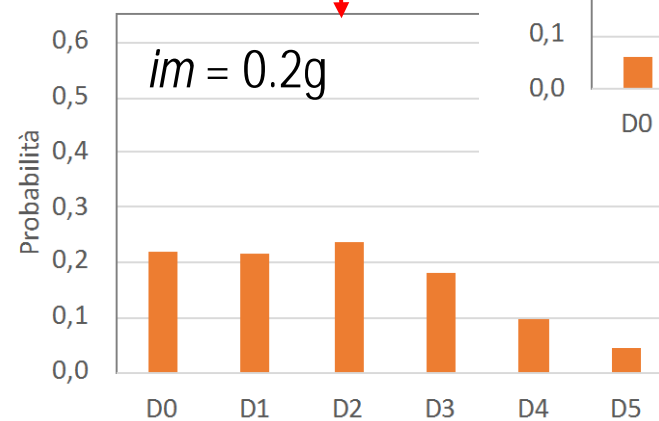
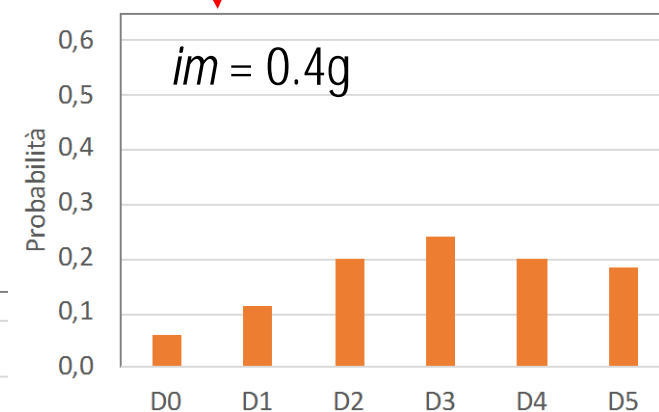


$$IM_{D3} = 0.3g$$

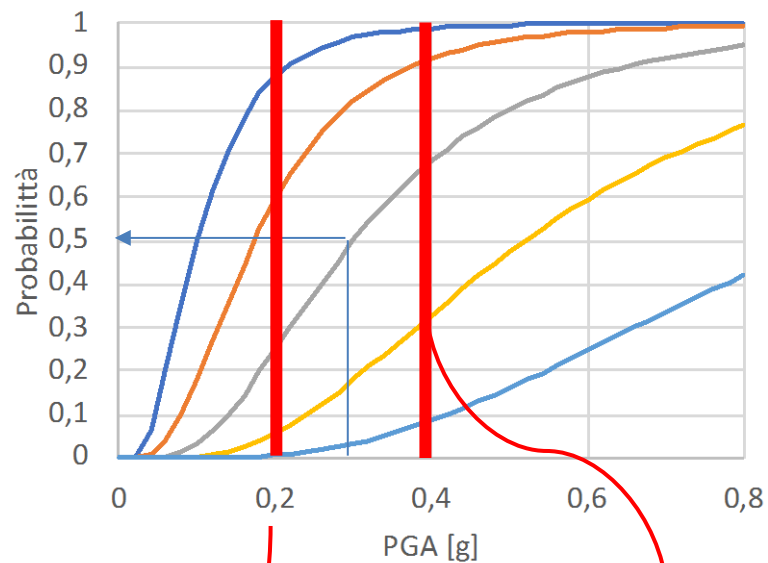
$$\beta_{D3} = 0.9$$

$$IM_{D1} = 0.1g$$

$$IM_{D5} = 0.9g$$



FRAGILITY FUNCTIONS – influence of β

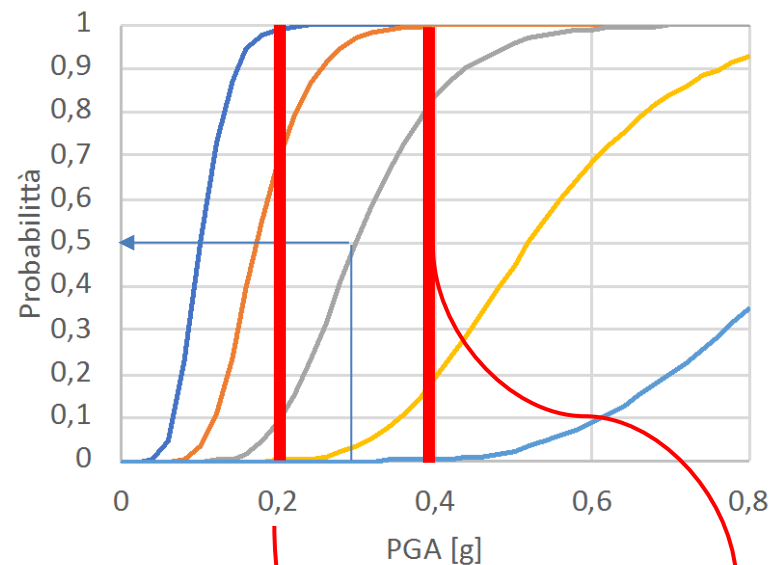
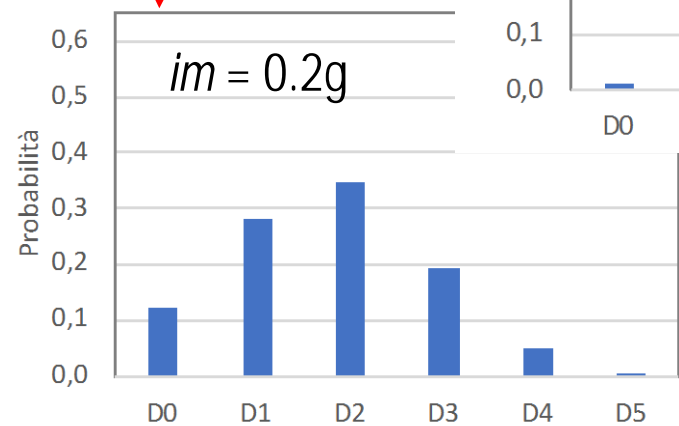
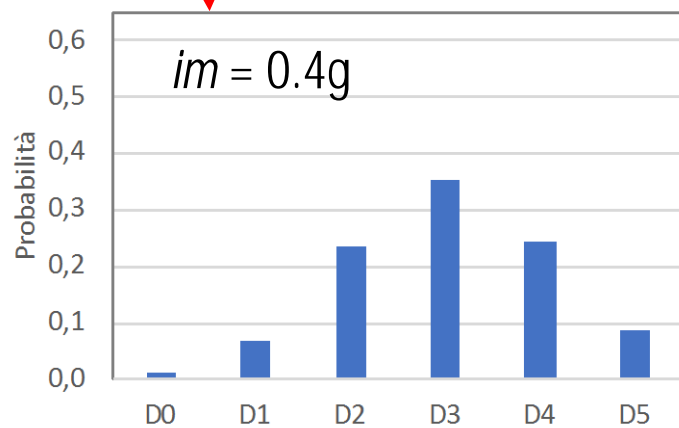


$$IM_{D3} = 0.3g$$

$$\beta_{D3} = 0.6$$

$$IM_{D1} = 0.1g$$

$$IM_{D5} = 0.9g$$

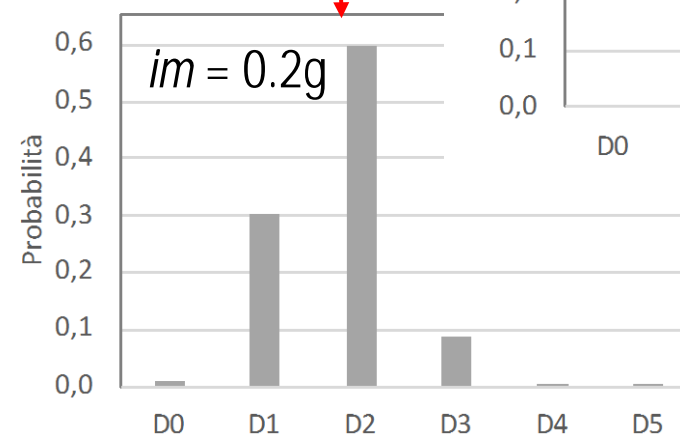
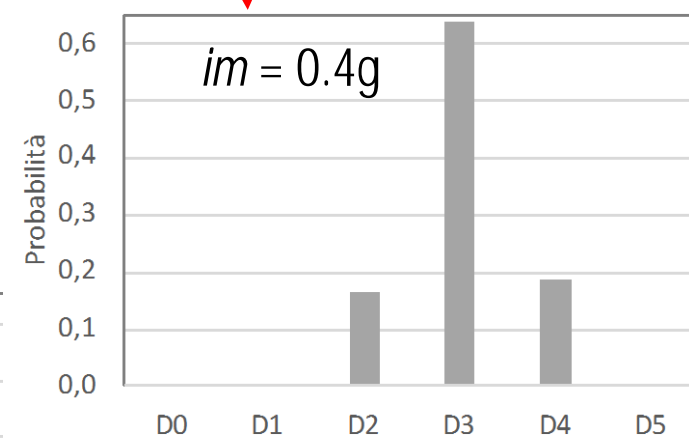


$$IM_{D3} = 0.3g$$

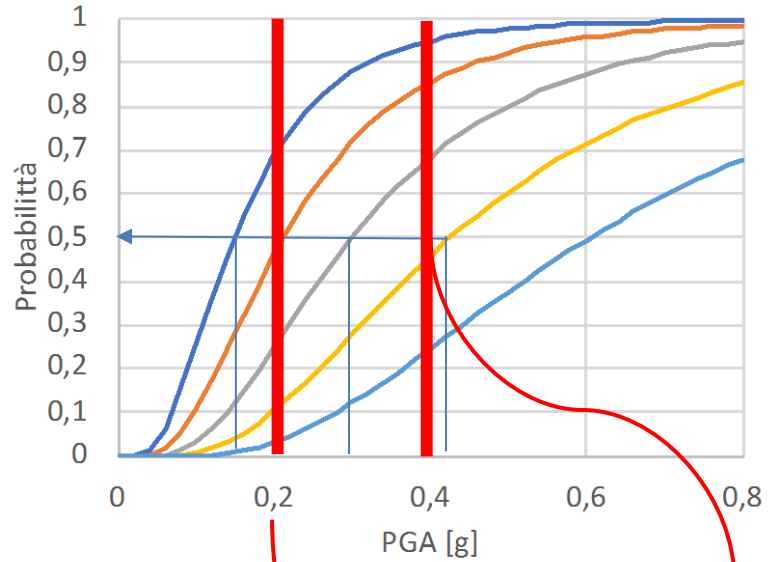
$$\beta_{D3} = 0.3$$

$$IM_{D1} = 0.1g$$

$$IM_{D5} = 0.9g$$



FRAGILITY FUNCTIONS – influence of the ductility

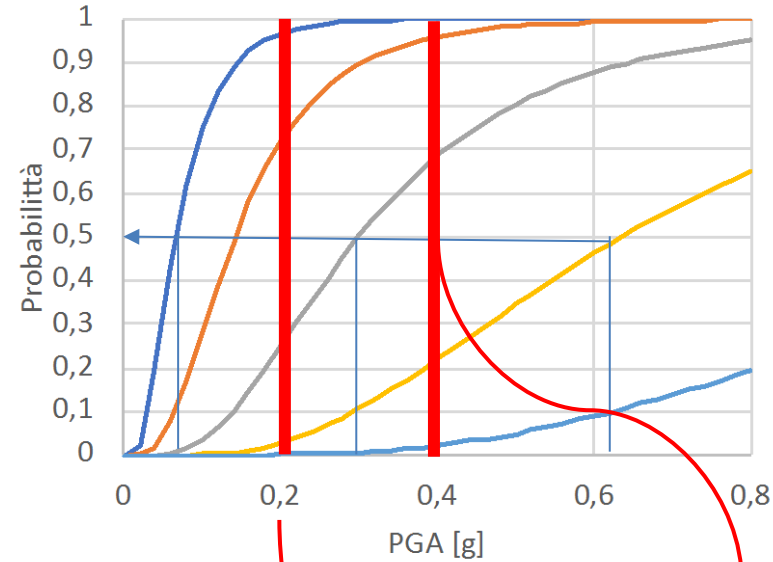
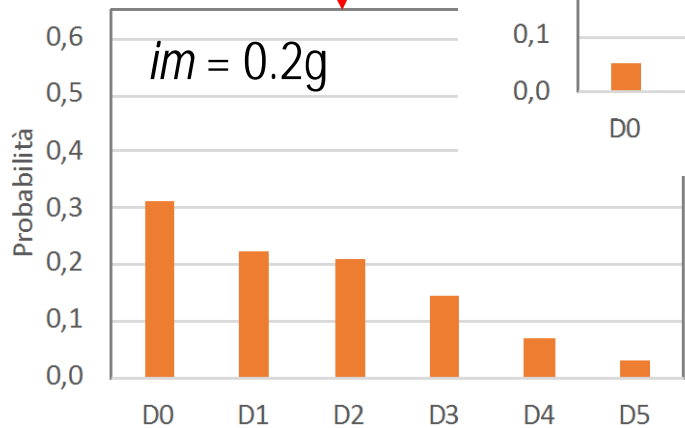
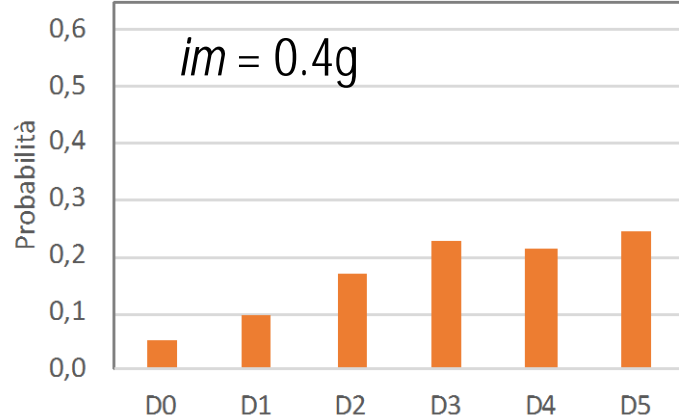


$$IM_{D3} = 0.3g$$

$$IM_{D1} = 0.15g$$

$$IM_{D4} = 0.43g$$

$$\beta_{D3} = 0.6$$

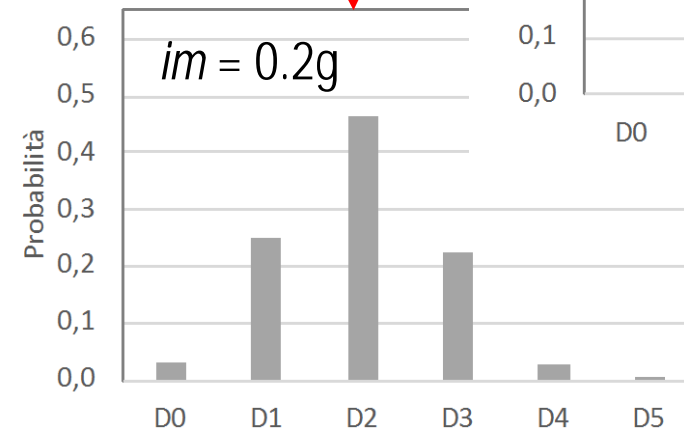
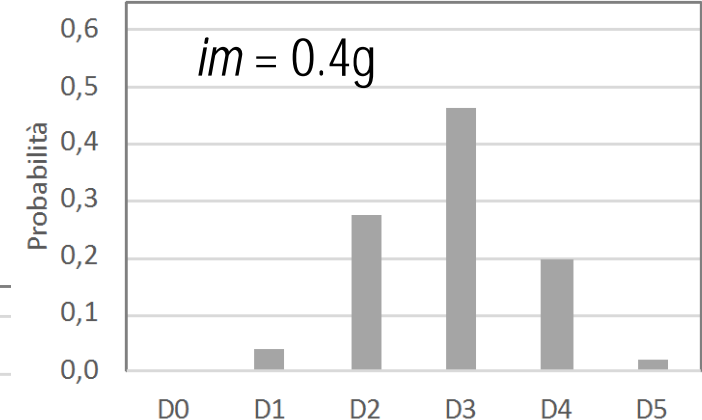


$$IM_{D3} = 0.3g$$

$$IM_{D1} = 0.07g$$

$$IM_{D4} = 0.64g$$

$$\beta_{D3} = 0.6$$



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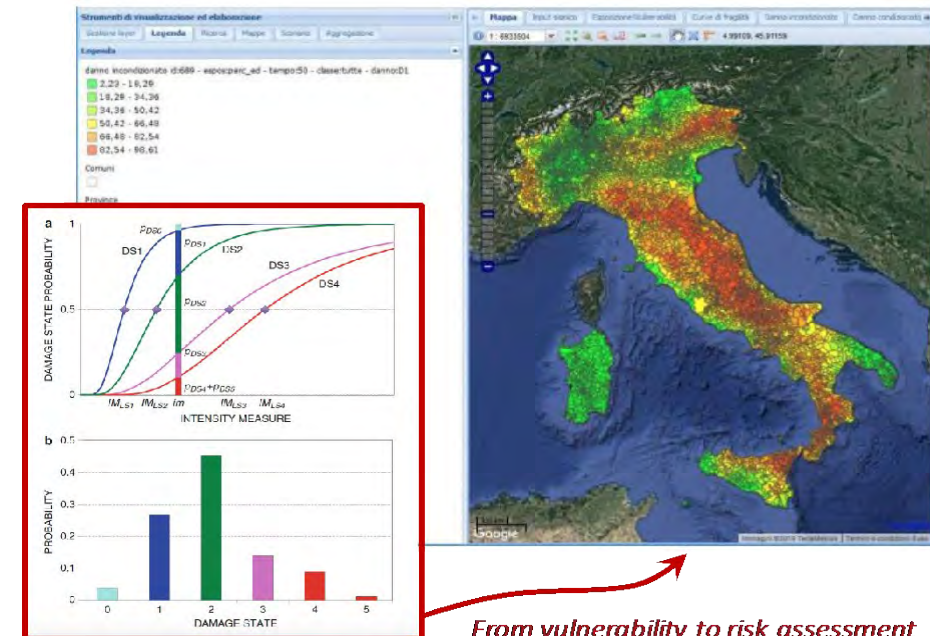
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FRAGILITY CURVES: *how are they obtained?*

- ☐ **Empirical / Observational**
- ☐ **Expertise-based / Heuristic**
- ☐ **Mechanical-based**
- ☐ **Hybrid methods**

Critical issues on:

- the incompleteness/reliability of empirical data and the definition of a robust damage metrics (Empirical/Observational)
- the representativeness of archetype buildings and the need of calibration & validation (Mechanical-based)
-

References for the classification of methods:

- Rossetto T., D'Ayala D., Ioannou I., Meslem A. (2014) Evaluation of existing fragility curves ,
- Chapter 3 In SYNER-G: Typology Definition and Fragility Functions for Physical Elements at Seismic Risk: Elements at Seismic Risk, Geotechnical, Geological and Earthquake Engineering 27 pp. 420

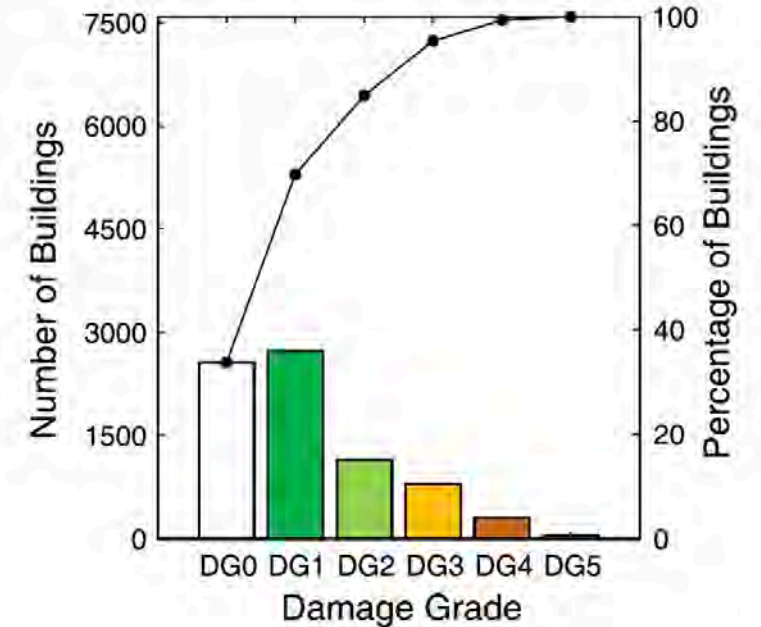
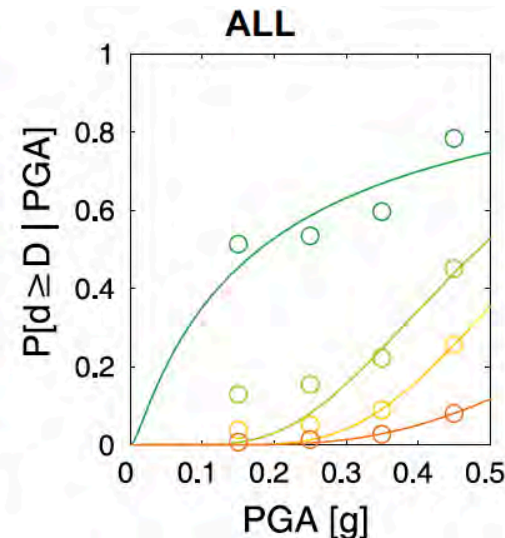
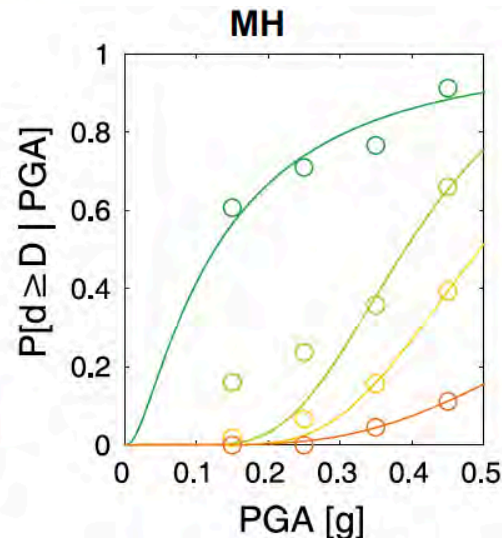
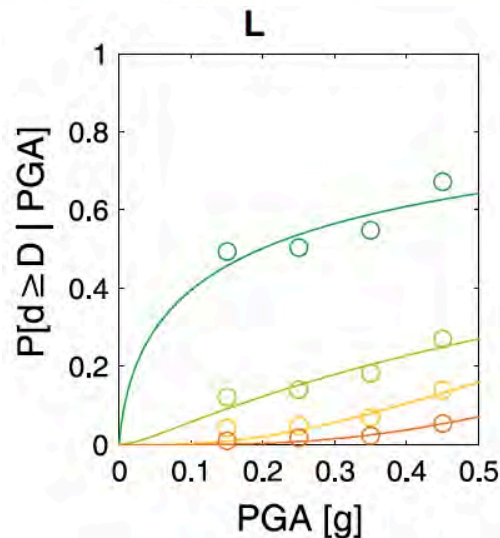
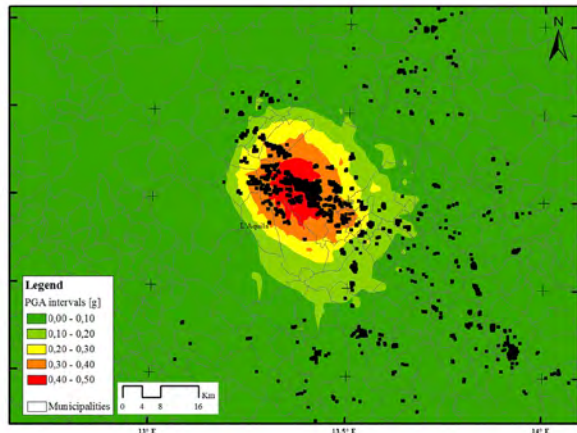
❑ Empirical / Observational

- ✓ Derived from observed damage, by a direct correlation with the intensity measure
- ✓ Empirical data are usually referred to macroseismic intensity, which is not an instrumental measure, but recently fragility curves are also derived directly in terms of PGA, thanks to the use of shake-maps
- ✓ Empirical data should represent the actual seismic behavior of buildings and can be very useful also for the validation of the others models
- ✓ Vulnerability is dependent on the local structural features of buildings, so the extrapolation of empirical fragility functions to other areas is questionable, in particular for traditional masonry buildings

FRAGILITY CURVES: *how are they obtained?*

□ Empirical / Observational – *Examples for RC structures*

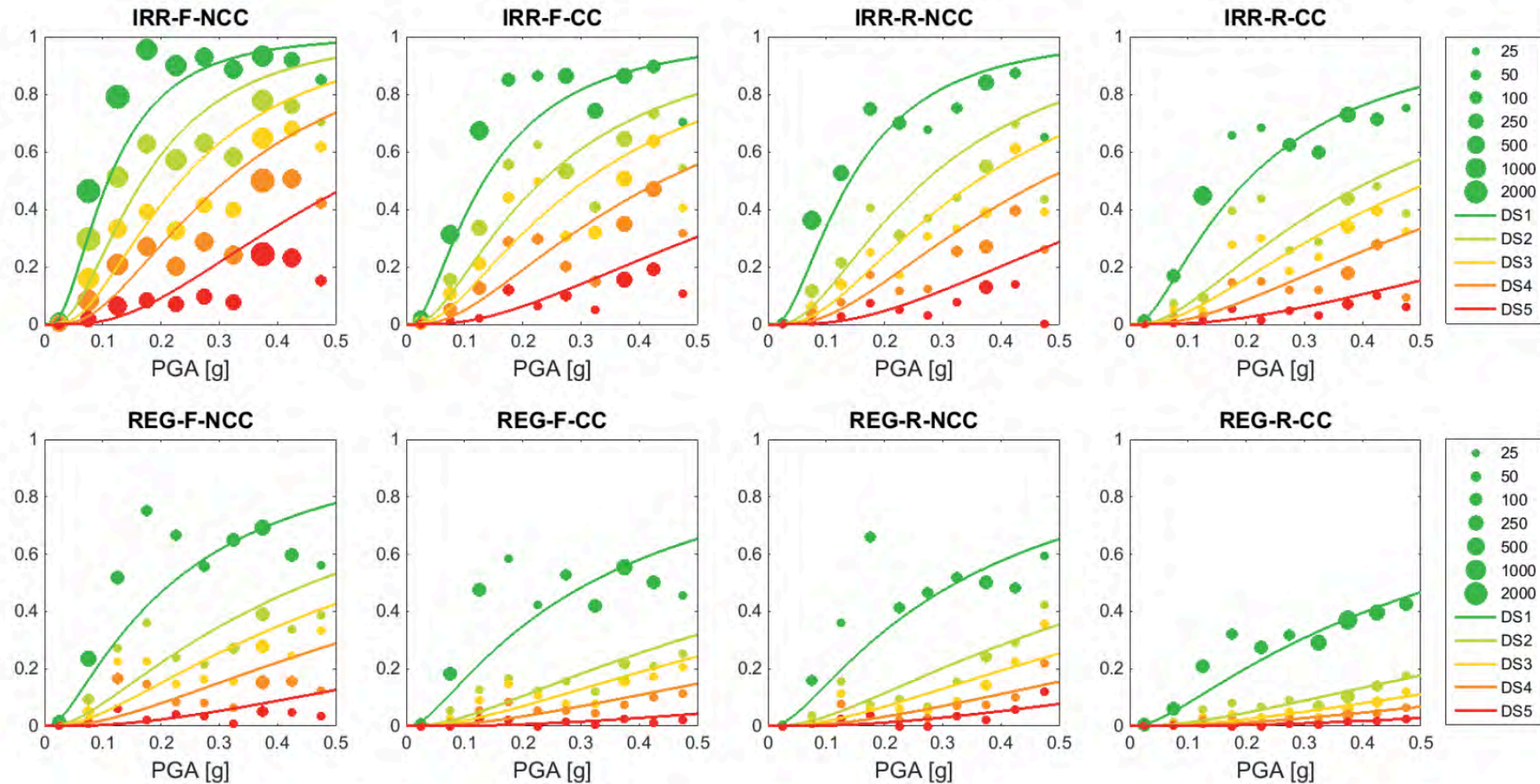
L'AQUILA 2009 earthquake: derivation of fragility curves for sub-typologies



FRAGILITY CURVES: *how are they obtained?*

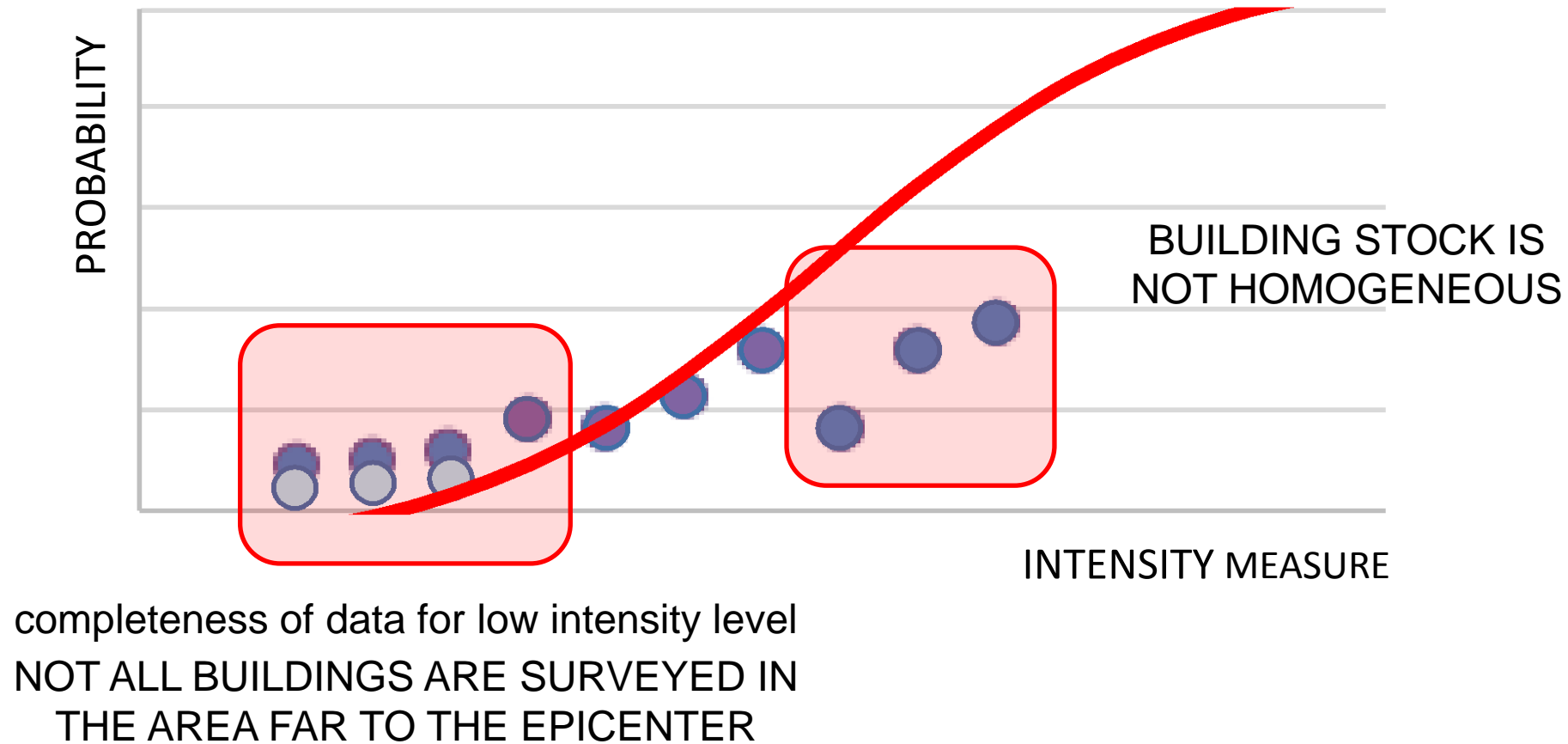
□ Empirical / Observational – *Examples for URM structures*

L'AQUILA 2009 earthquake: derivation of fragility curves for sub-typologies



FRAGILITY CURVES: *how are they obtained?*

❑ Empirical / Observational – *some critical issues*



❑ Mechanical-based

Increasing computational effort

- Analytical simplified
- Numerical by nonlinear static analyses
- Numerical by nonlinear dynamic analyses

key features of the building class (structural system, geometry, material properties) are quantified (median values, dispersion)

archetype buildings are identified and modelled in detail; dispersion of parameters are related to the whole building stock and not to the uncertainties of the single building

FRAGILITY CURVES: *how are they obtained?*

❑ Expertise-based / Heuristic – Expert elicitation

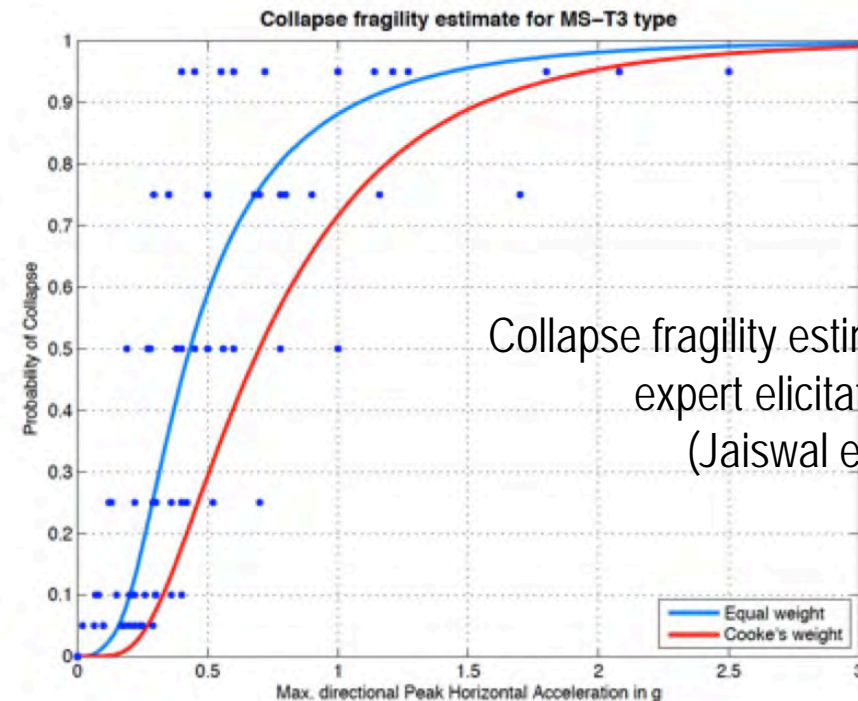
- ✓ Expert elicitation is used to assess vulnerability of building types, if no data is available and structural analysis is not feasible; one or more experts can offer an opinion on the level of demand at which damage is likely to occur.
- ✓ To process expert judgments the Delphi method (Dalkey, 1969) or the Cooke's method (Cooke, 1991) can be used.

Lisbon workshop, September 23, 2012 (Jaiswal et al 2013)

Organized by U.S. Geological Survey's Prompt Assessment for Global Earthquakes Response (PAGER) and Global Earthquake Model (GEM) -
Expert solicitation to develop DPM for 20 building classes, after checking the reliability of experts by seed questions



Experts responding to target questions at the workshop



Collapse fragility estimates obtained using expert elicitation process.
(Jaiswal et al 2013)

MACROSEISMIC MODEL – Lagomarsino & Giovinazzi 2006

- ❑ Classifiable as **Expertise-based / Heuristic**
- ❑ Derived from the **European Macroseismic Scale** (Grunthal 1998), which defines six vulnerability classes (from A to F) and various building types (seven of them related to masonry buildings).

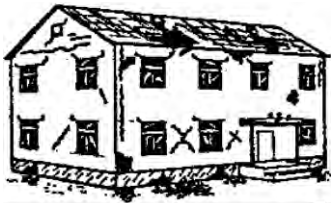
EMS 98



GRADE 1:
Negligible to slight damage



GRADE 2:
Moderate damage



GRADE 3:
Substantial to heavy damage



GRADE 4:
Very heavy damage



GRADE 5:
Destruction

Type of Structure		Vulnerability Class					
		A	B	C	D	E	F
MASONRY	rubble stone, fieldstone	O					
	adobe (earth brick)	O	H				
	simple stone	I	O				
	massive stone			H	H		
	unreinforced, with manufactured stone units	I	O	I			
	unreinforced, with RC floors			H	H		
	reinforced or confined				I	O	
REINFORCED CONCRETE (RC)	frame without earthquake-resistant design (ERD)	I	O	I			
	frame with moderate level of ERD			I	O	H	
	frame with high level of ERD				I	O	H
	walls without ERD			I	O		
	walls with moderate level of ERD			I	O		
	walls with high level of ERD				I	O	
						I	O

O most likely vulnerability class. — probable range.
--- range of less probable, exceptional cases

*There isn't a direct
correspondance between a
specific structural typology
& a vulnerability class*

MACROSEISMIC MODEL – Lagomarsino & Giovinazzi 2006

- ❑ For each building class, the linguistic definitions of EMS98 may be translated in quantitative terms, by the fuzzy set theory, and an incomplete **Damage Probability Matrix** (DPM) is obtained.
- ❑ The completion is made by using the **binomial probability distribution**.

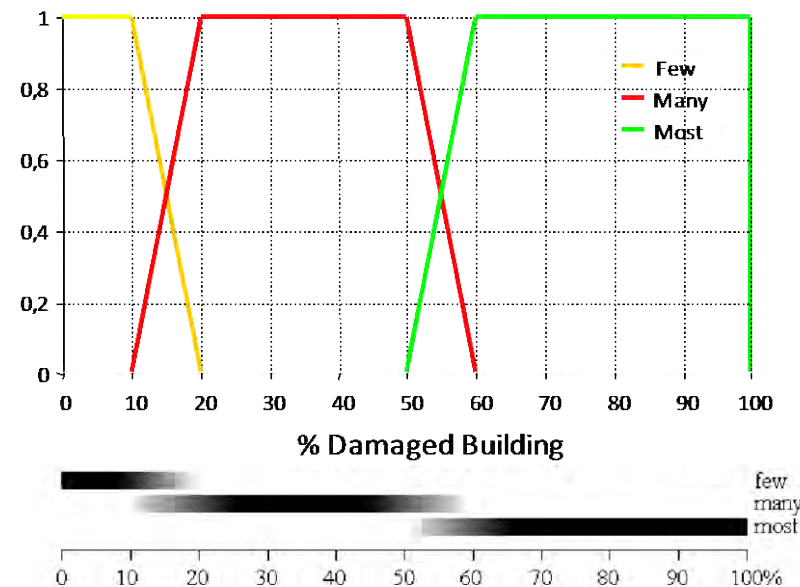
EMS 98

D_k/I	$k=0$ (negligible to slight)	$k=1$ (moderate)	$k=2$ (substantial to heavy)	$k=3$ (very heavy)	$k=4$ (destruction)
5	Few A or B				
6	Many A or B, Few C	Few A or B			
7		Many B, Few C	Many A, Few B	Few A	
8		Many C, Few D	Many B, Few C	Many A, Few B	Few A
9		Many D, Few E	Many C, Few D	Many B, Few C	Many A, Few B
10		Many E, Few F	Many D, Few E	Many C, Few D	Most A, Many B, Few C
11		Many F	Many E, Few F	Most C, Many D, Few E	Most B, Many C, Few D
12					All A or B, Nearly All C, Most D or E or F

CLASS B						
I	D_0	D_1	D_2	D_3	D_4	D_5
V	All-Few	Few	None	None	None	None
VI	Many + 7/3Few	Many	Few	None	None	None
VII	7/3Few	Many	Many	Few	None	None
VIII	1/3Few	2Few	Many	Many	Few	None
IX	None	1/3Few	2Few	Many	Many	Few
X	None	None	1/3Few	2Few	Many+ Few	Many
XI	None	None	None	Nearly Few	8/3Few	Most
XII	None	None	None	None	None	All

quantified

FUZZY SET THEORY

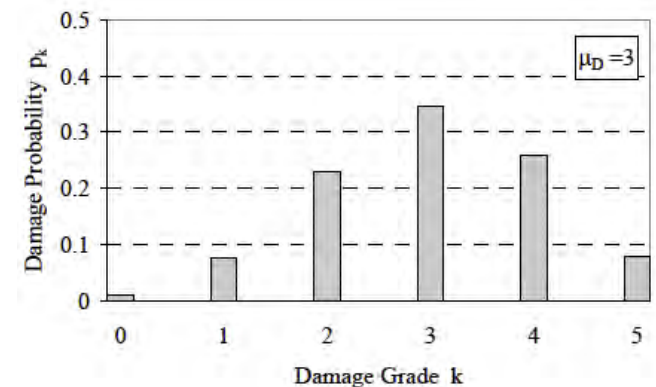


completed

BINOMIAL PROBABILITY DISTRIBUTION

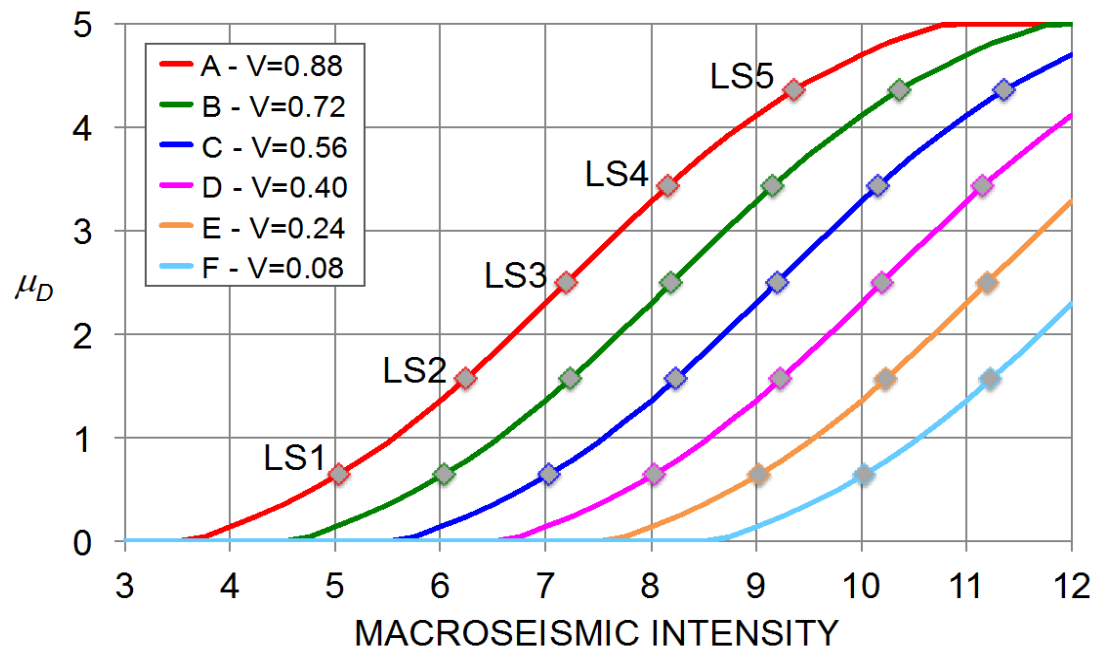
$$\text{PMF: } p_k = \frac{5!}{k! (5-k)!} \left(\frac{\mu_D}{5} \right)^k \left(1 - \frac{\mu_D}{5} \right)^{5-k}$$

μ_D mean damage of distribution



MACROSEISMIC MODEL – Lagomarsino & Giovinazzi 2006

- ❑ For each building class, the linguistic definitions of EMS98 may be translated in quantitative terms, by the fuzzy set theory, and an incomplete **Damage Probability Matrix** (DPM) is obtained.
- ❑ The completion is made by using the **binomial probability distribution**.
- ❑ The vulnerability is analitcally expressed by a **curve** (Bernardini et al. 2011), which gives the mean damage μ_D as a function of the macroseismic intensity I



$$\mu_D = 2.5 + 3 \tanh \left(\frac{I + 6.25V - 12.7}{Q} \right) \quad (0 \leq \mu_D \leq 5)$$

The curve is defined by two parameters representative of the seismic behavior of a group of homogeneous buildings: the vulnerability index **V** and the ductility index **Q**

Class	A	B	C	D	E
V	0.9	0.74	0.58	0.42	0.26

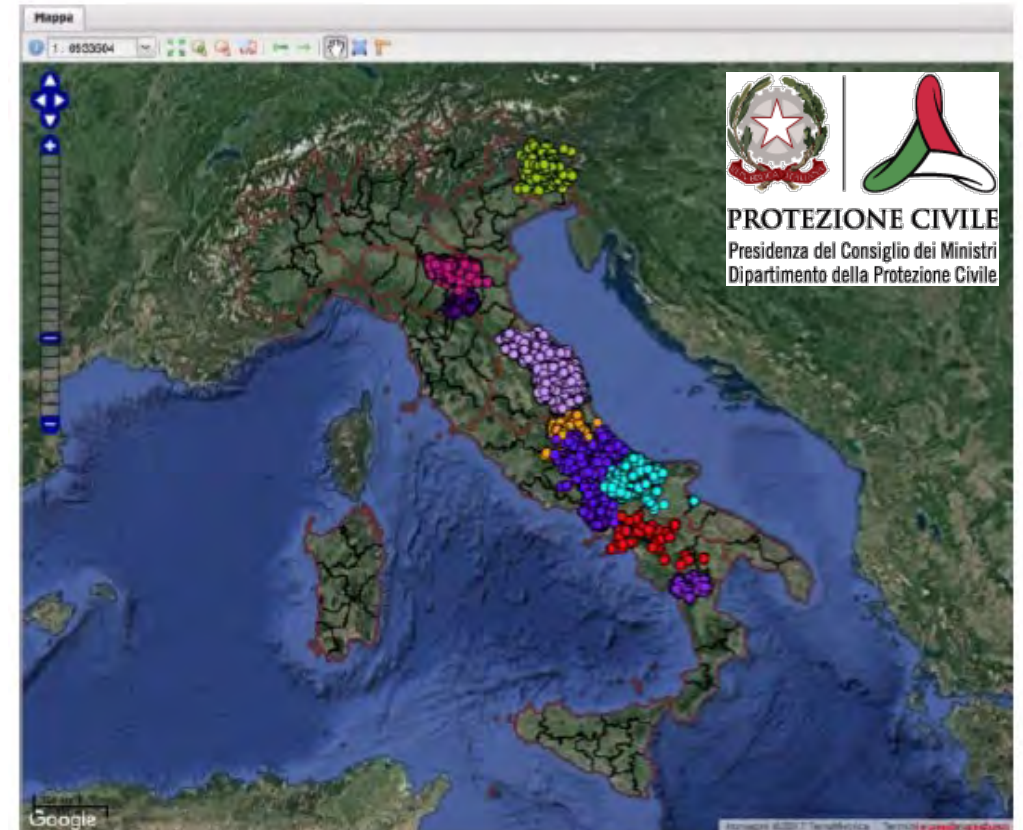
Q assumed costant and equal to 2.3 for residential buildings

Calibration of the macroseismic model by D.A.D.O. Database

The macroseismic model has been recently **calibrated by the observed damage**, collected after many earthquakes in Italy in the database Da.D.O. developed by the Italian Department of Civil Protection (DPC)

Evento	Anno	Record	Vers.scheda
Friuli '76	1976	41.852	Friuli '76
Irpinia '80	1980	38.079	Irpinia '80
Abruzzo '84	1984	51.817	Abruzzo '84
Umbria Marche '97	1997	48.525	AeDES 09/97
Pollino '98	1998	17.442	AeDES 06/98
Molise Puglia 2002	2002	24.141	AeDES 05/2000
Emilia 2003	2003	1011	AeDES 05/2000
L'aquila 2009	2009	74.049	AeDES 06/2008
Emilia 2012	2012	22.554	AeDES 06/2008
Totale		319.470	

DaDO database: more than 300000 buildings surveyed after 9 different earthquakes in Italy since Friuli 1976



REF: Dolce M., Speranza E., Giordano F., Borzi B., Bocchi F., Conte C., Di Meo A., Faravelli M., Pascale V. (2019) Observed damage database of past Italian earthquakes: the Da.D.O. Webgis. Bollettino di Geofisica Teorica e Applicata 60 (2) 141-164.

DaDO database: more than 300000 buildings surveyed after 9 different earthquakes in Italy since Friuli 1976

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Presidenza del Consiglio dei Ministri
Dipartimento della Protezione Civile

CONFERENZA DELLE REGIONI E DELLE
PROVINCE AUTONOME



SCHEDA DI 1° LIVELLO DI RILEVAMENTO DANNO, PRONTO INTERVENTO E AGIBILITÀ PER EDIFICI ORDINARI NELL'EMERGENZA POST-SISMICA (AeDES 06/2008)

Codice Richiesta

DAMAGE SECTION

SEZIONE 4 Danni ad ELEMENTI STRUTTURALI e provvedimenti di pronto intervento (P.I.) eseguiti

Livello - estensione	Componente strutturale - Danno preesistente	DANNO ⁽¹⁾										PROVEDIMENTI DI P.I. ESEGUITI						
		D4-D5 Gravissimo			D2-D3 Medio grave			D1 Leggero			Nullo	Nessuno	Demolizioni	Cerchiature e/o tiranti	Riparazione	Puntelli	Trasferenze e protezione passaggi	
		> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3								
		A	B	C	D	E	F	G	H	I								L
1	Strutture verticali	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
2	Solai	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
3	Scale	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
4	Copertura	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
5	Tamponature-tramezzi	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
6	Danno preesistente	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>

(1) - Di ogni livello di danno indicare l'estensione solo se esso è presente. Se l'oggetto indicato nella riga non è danneggiato campire **Nullo**.

(1) - Di ogni livello di danno indicare l'estensione solo se esso è presente. Se l'oggetto indicato nella riga non è danneggiato campire **Nullo**.

TYOPOLOGICAL SECTION with valuable information also on vulnerability

SEZIONE 3 Tipologia (multiscelta; per gli edifici in muratura indicare al massimo 2 tipi di combinazioni strutture verticali-solai)

<div>Strutture verticali</div> <div>Strutture orizzontali</div>	Strutture verticali	Non identificate	Strutture in muratura								Altre strutture				
			A tessitura irregolare e di cattiva qualità (Pietrame non squadrato, cordoli,...)				A tessitura regolare e di buona qualità (Blocchi; mattoni; pietra squadrata,...)				Piastrì isolati	Mista	Rinforzata	Telai in c.a.	<input type="checkbox"/>
			Senza catene o cordoli		Con catene o cordoli		Senza catene o cordoli		Con catene o cordoli					Pareti in c. a.	<input type="checkbox"/>
			Senza catene o cordoli		Con catene o cordoli		Senza catene o cordoli		Con catene o cordoli					Telai in acciaio	<input type="checkbox"/>
A	B	C	D	E	F	G	H								
1	Non identificate	<input type="radio"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	SI	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>			
2	Volte senza catene	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	G1	H1	<input type="checkbox"/>			
3	Volte con catene	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>			
4	Travi con soletta deformabile (travi in legno con semplice fasciato, travi e volte,...)	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	NO	G2	H2	<input type="checkbox"/>		
5	Travi con soletta semirigida (travi in legno con doppio fasciato, travi e tavelloni,...)	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
6	Travi con soletta rigida (solai di c.a., travi ben collegate a solette di c.a.,...)	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	G3	H3	<input type="checkbox"/>		

REGOLARITA'			Non regolare	Regolare
			A	B
1	Forma pianta ed elevazione	<input type="radio"/>	<input type="checkbox"/>	<input type="checkbox"/>
2	Disposizione temporale	<input type="radio"/>	<input type="checkbox"/>	<input type="checkbox"/>

Copertura	
1	<input type="radio"/> Spingente pesante
2	<input type="radio"/> Non spingente pesante
3	<input type="radio"/> Spingente leggera
4	<input type="radio"/> Non spingente leggera

Copertura

1 ☐ Spingente pesante
2 ☐ Non spingente pesante
3 ☐ Spingente leggera
4 ☐ Non spingente leggera

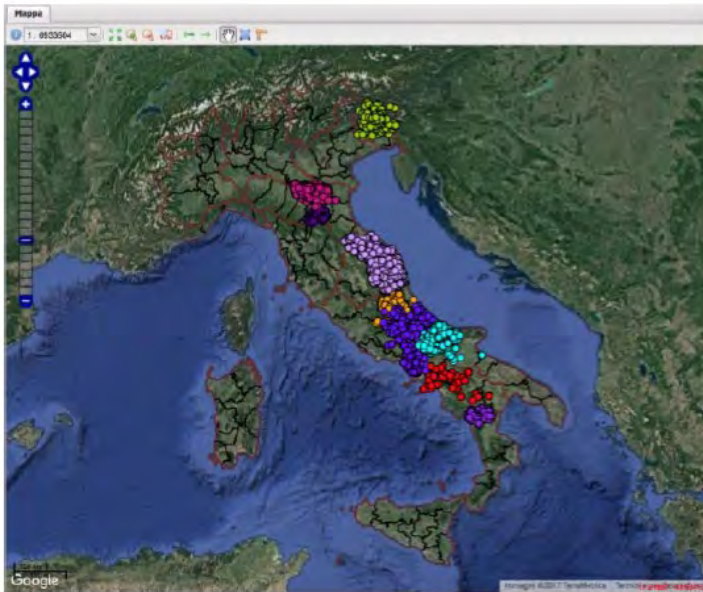
MACROSEISMIC MODEL – Recent developments made by UNIGE



FIRST STEP: Conversion of damage data of AeDES forms into a DAMAGE LEVEL compatible with that defined at global scale according to the EMS98

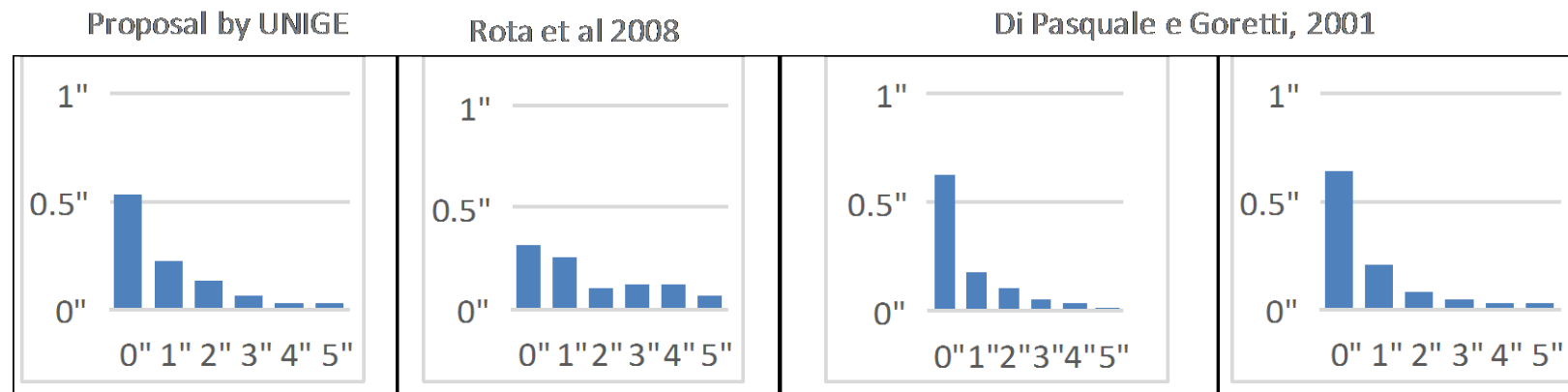
Different proposals:

- ☐ Rota et al. 2008
- ☐ Pasquale and Goretti 2001
- ☐ D.A.D.O proposal by DPC
- ☐ Proposal by UNIGE



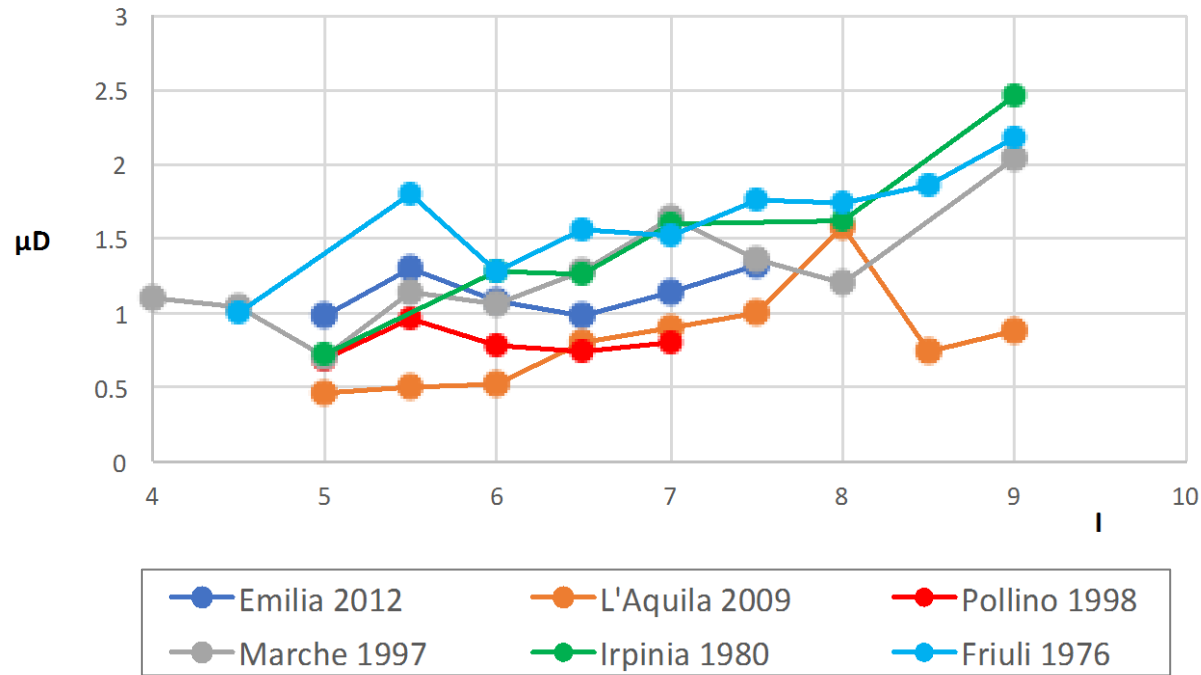
STATISTICAL
REPROCESSING OF DATA
AND DEFINITION OF DPM

DPM for $I=6.5$



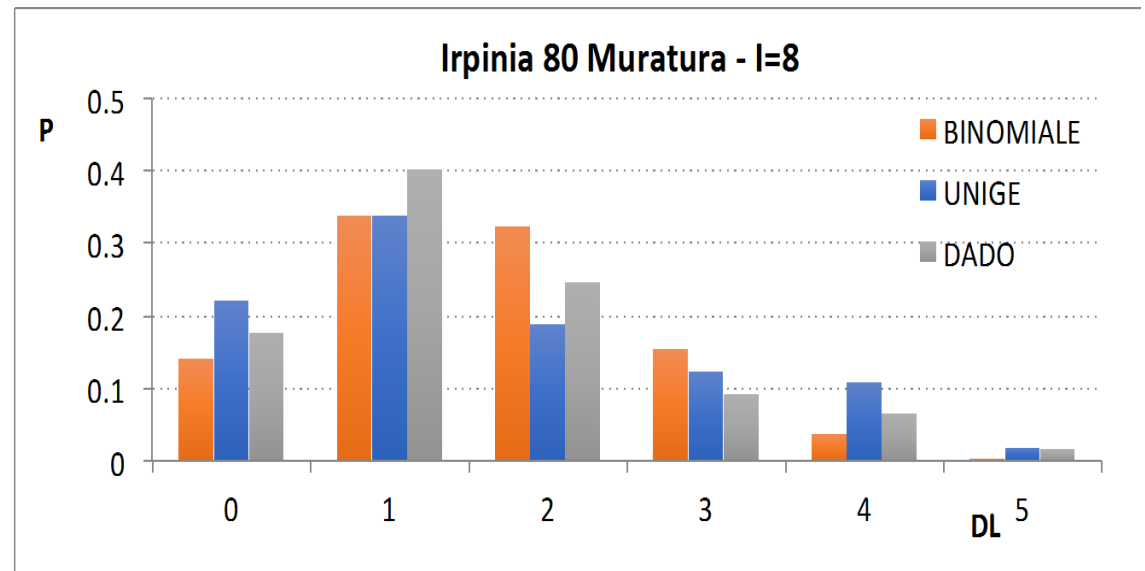
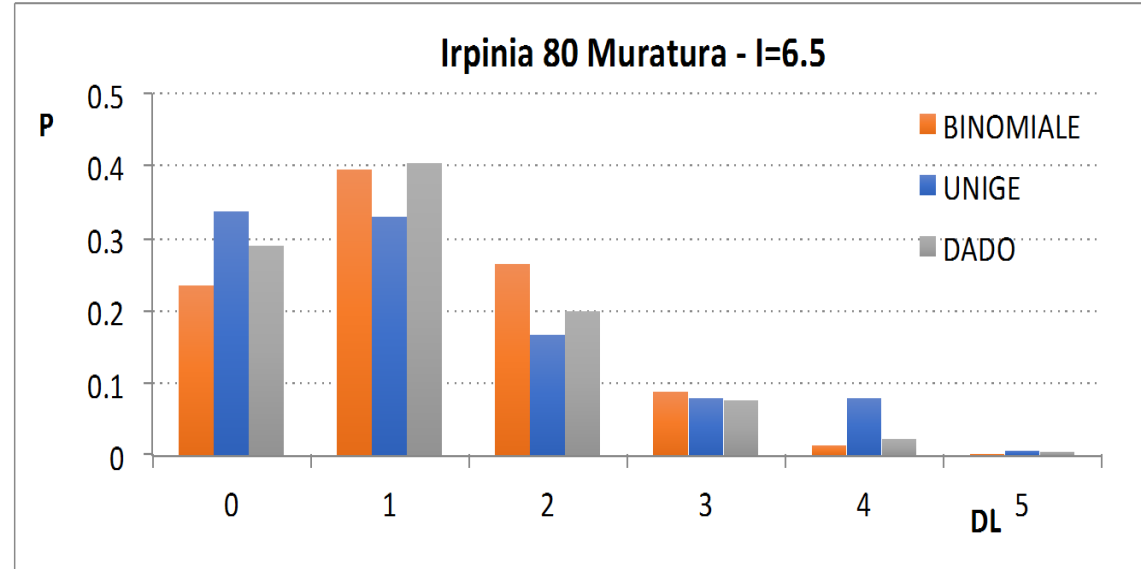
REF: Lagomarsino S., Cattari S., Ottonelli D. (2020) Macroseismic fragility curves for Italian residential URM buildings calibrated by observed damage , Bulletin of Earthquake Engineering, to be submitted.

MACROSEISMIC MODEL – Recent developments made by UNIGE

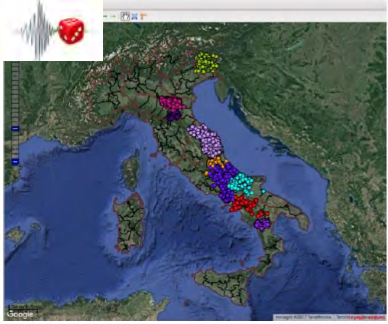


DATA from **Irpinia 1980** and **L'Aquila 2009** earthquakes are more robust and complete for the calibration

Evento Sismico	N° Edifici Iniziale	N° Edifici con Intensità	N° Edifici in Muratura
Friuli 1976	41852	41852	29641
Irpinia 1980	38079	33220	26335
Umbria-Marche 1997	48525	34873	29512
Pollino 1998	17442	16689	13887
L'Aquila 2009	74049	73793	51438
Emilia 2012	22554	22489	18194

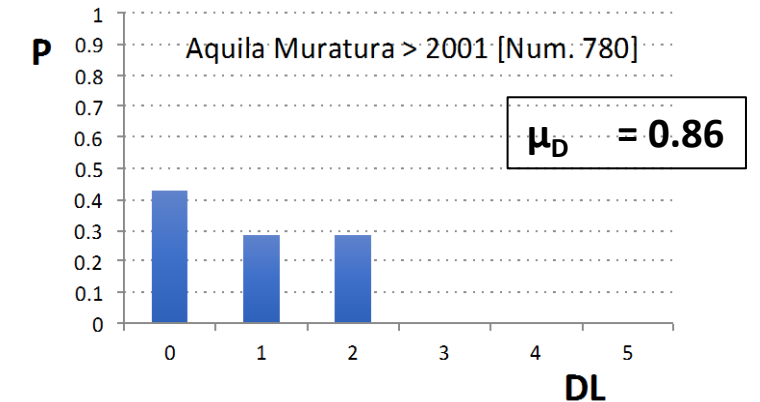
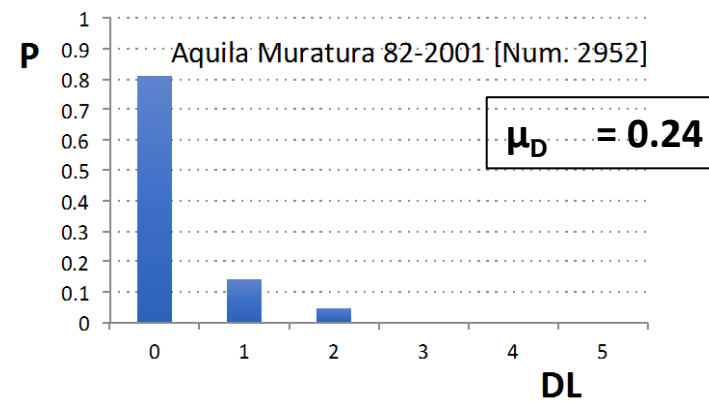
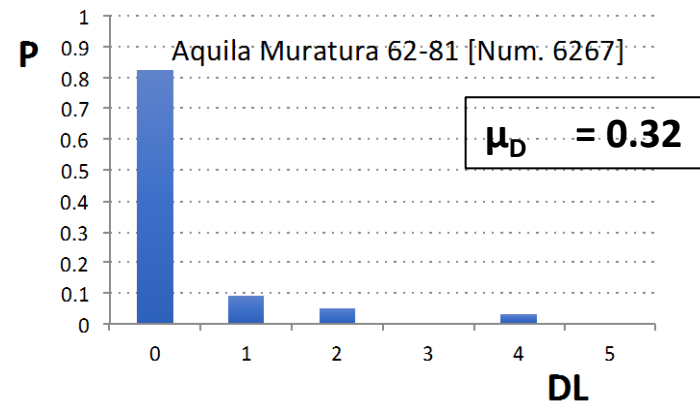
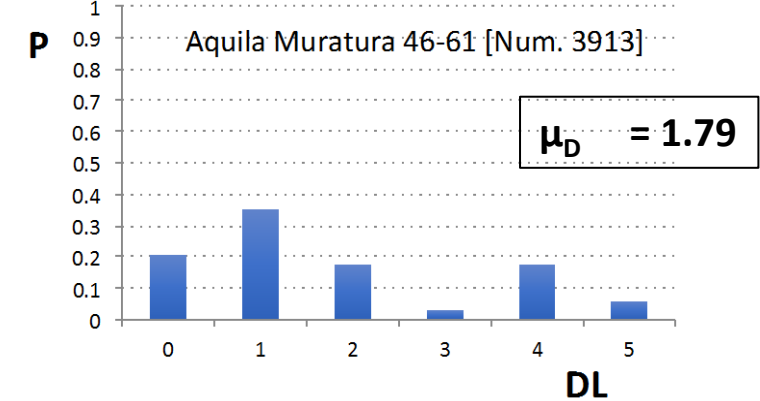
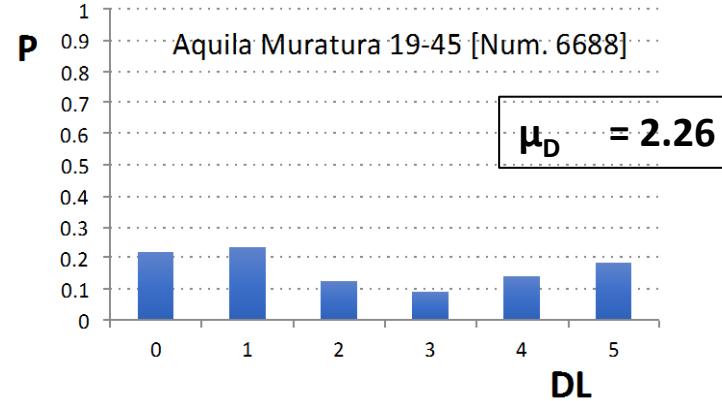
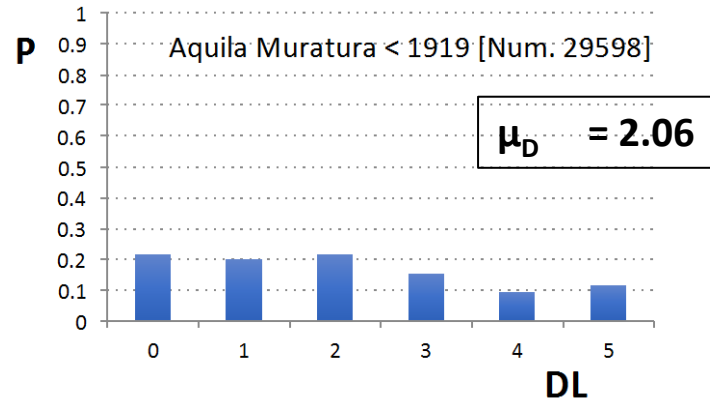


MACROSEISMIC MODEL – Recent developments made by UNIGE



The vulnerability of masonry buildings decreases significantly with the age of construction

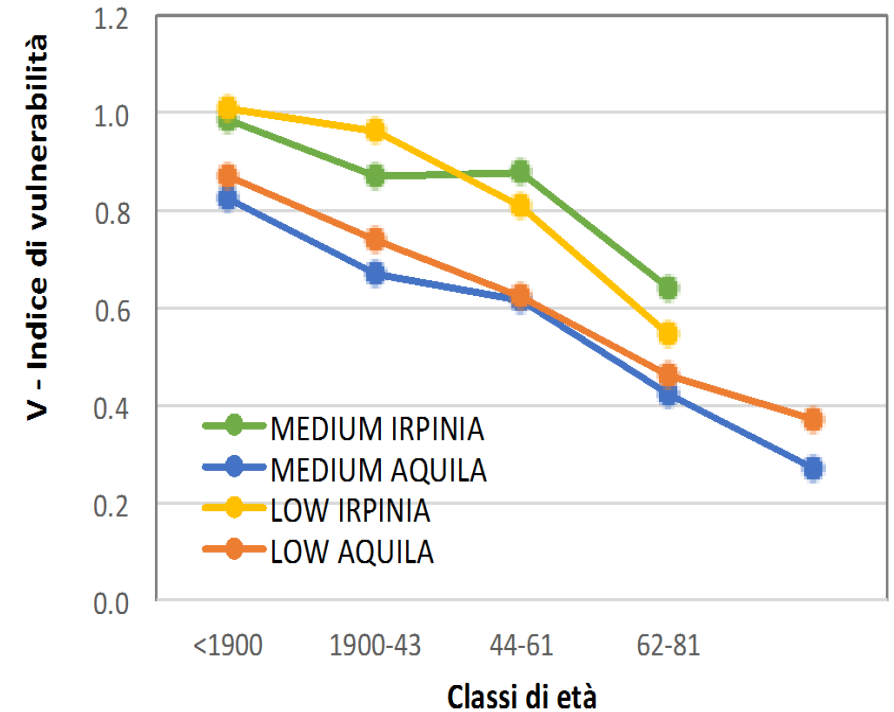
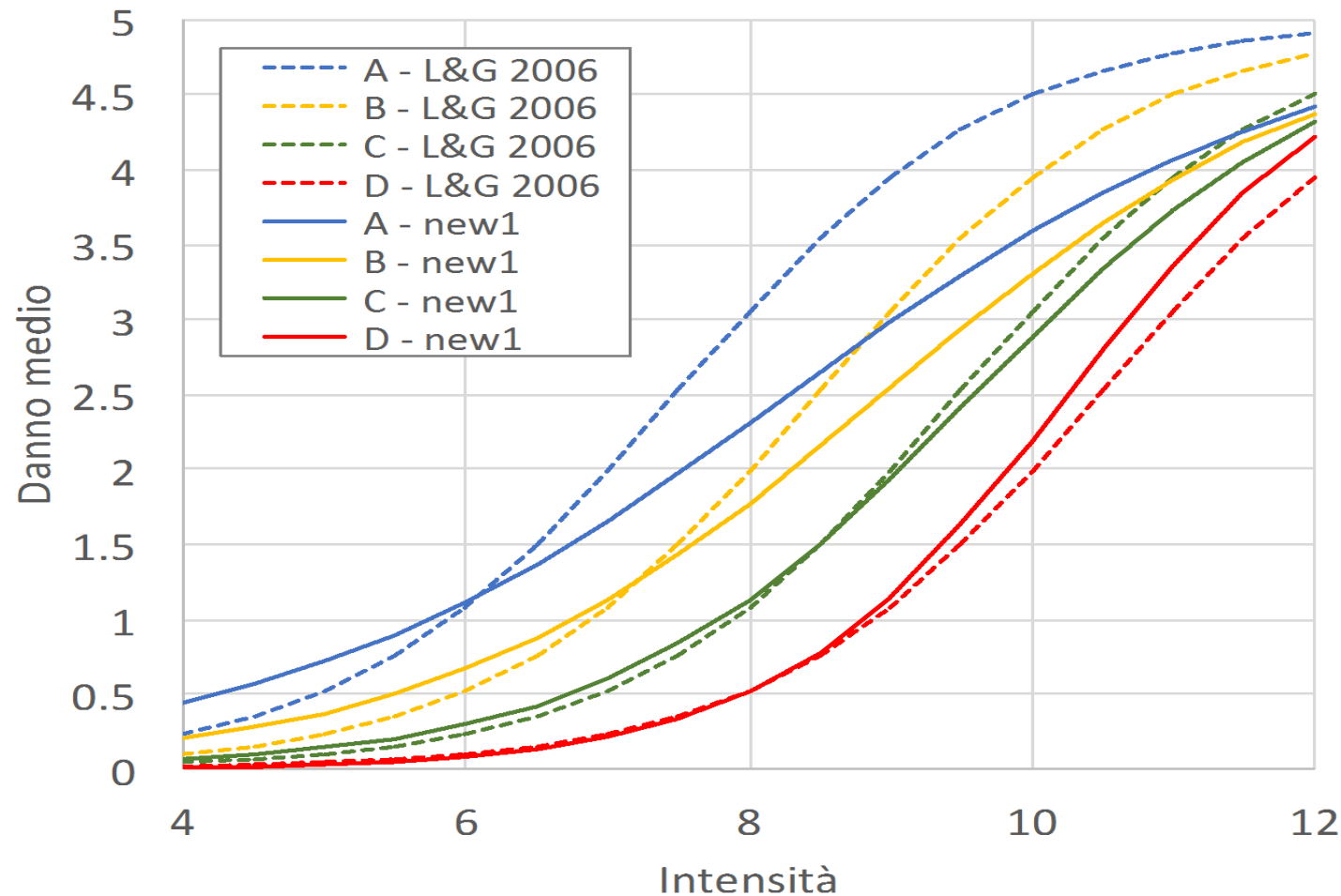
DAMAGE HISTOGRAM FOR L'AQUILA 2009 EARTHQUAKE VARYING THE AGE



MACROSEISMIC MODEL – Recent developments made by UNIGE

$$\mu_D = 2.5 \left[1 + \tanh \left(\frac{I + 3.45V - 11.7}{2.8V + 0.9} \right) \right]$$

Class	A	B	C	D
V	0.99	0.80	0.61	0.42

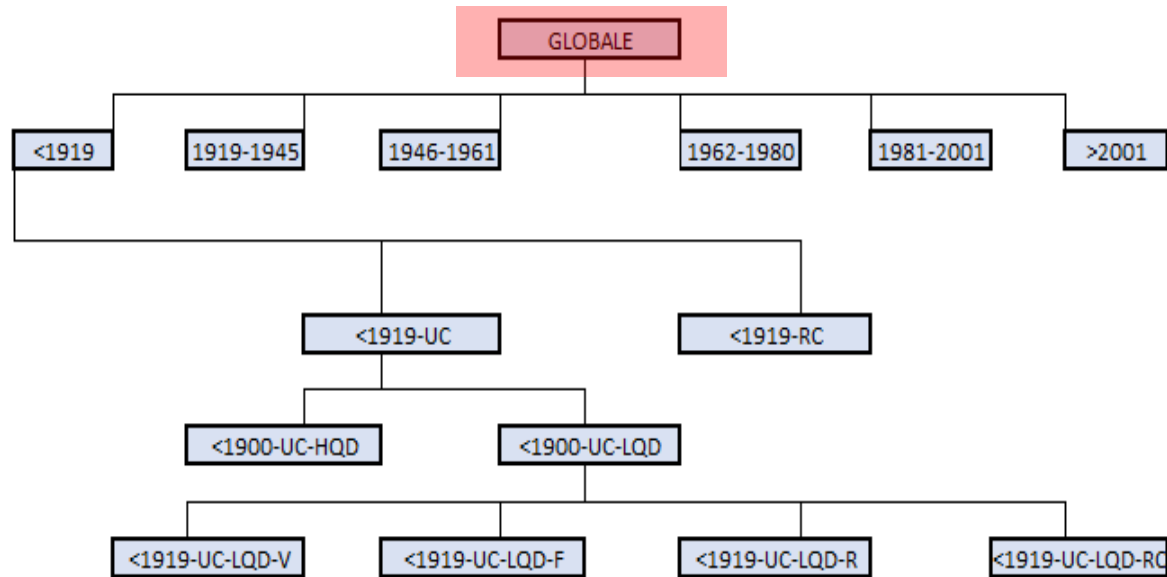


$$V = V_0 + \sum V_s$$

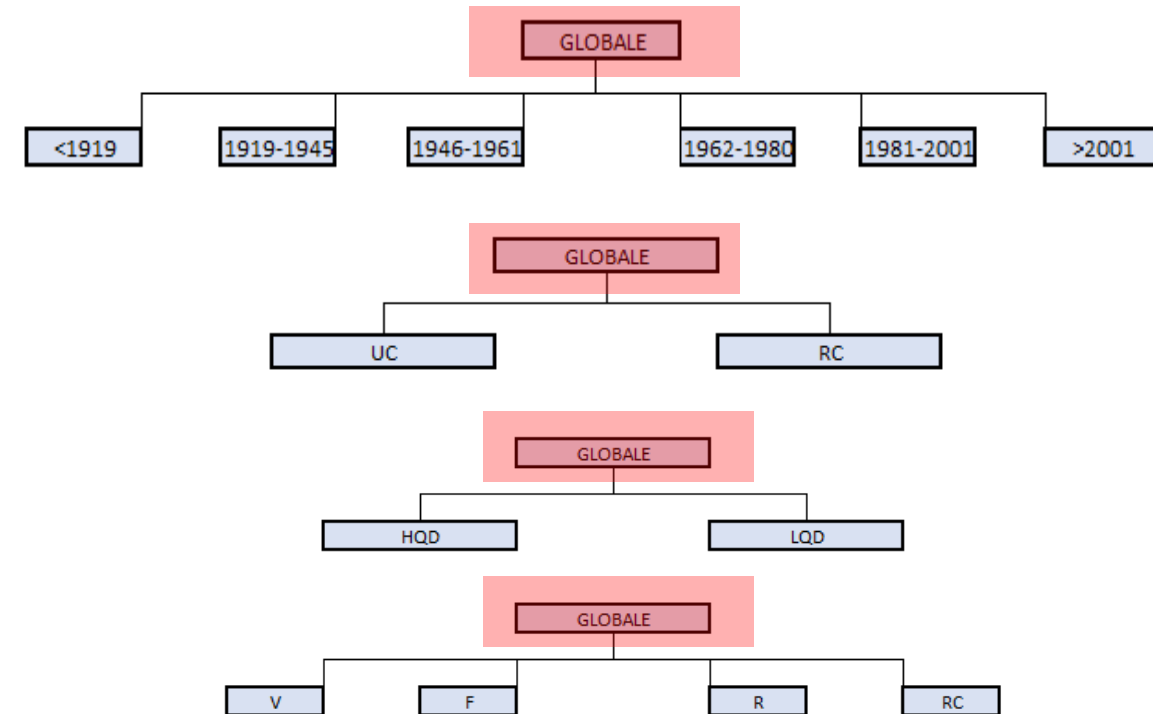
CALIBRATION AND COMBINATION OF BEHAVIOUR MODIFIERS

DERIVATION OF BEHAVIOUR MODIFIERS BY GROUPING THE DATA ON SPECIFIC SUB-CLASSES

Cascade process



Independent process



TAXONOMY of reference (Lagomarsino and Cattari 2013)

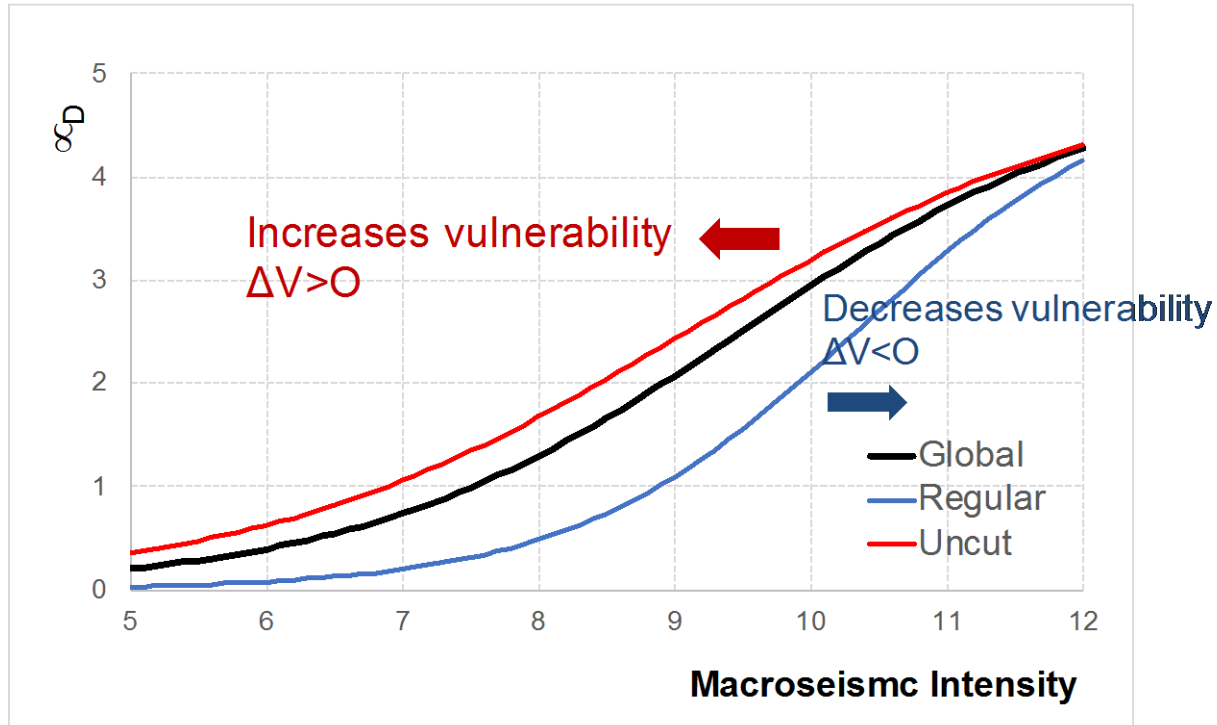
RC= Regular Cut masonry // UC=Uncut masonry

HQD=High Quality Details (tie rods & ring beams) // LQD=Low Quality Details (no tie rods / ring beams)

V=Vaults // F=Flexible Floor // R=Semi-rigid Floor // RC=Rigid Floor

CALIBRATION AND COMBINATION OF BEHAVIOUR MODIFIERS

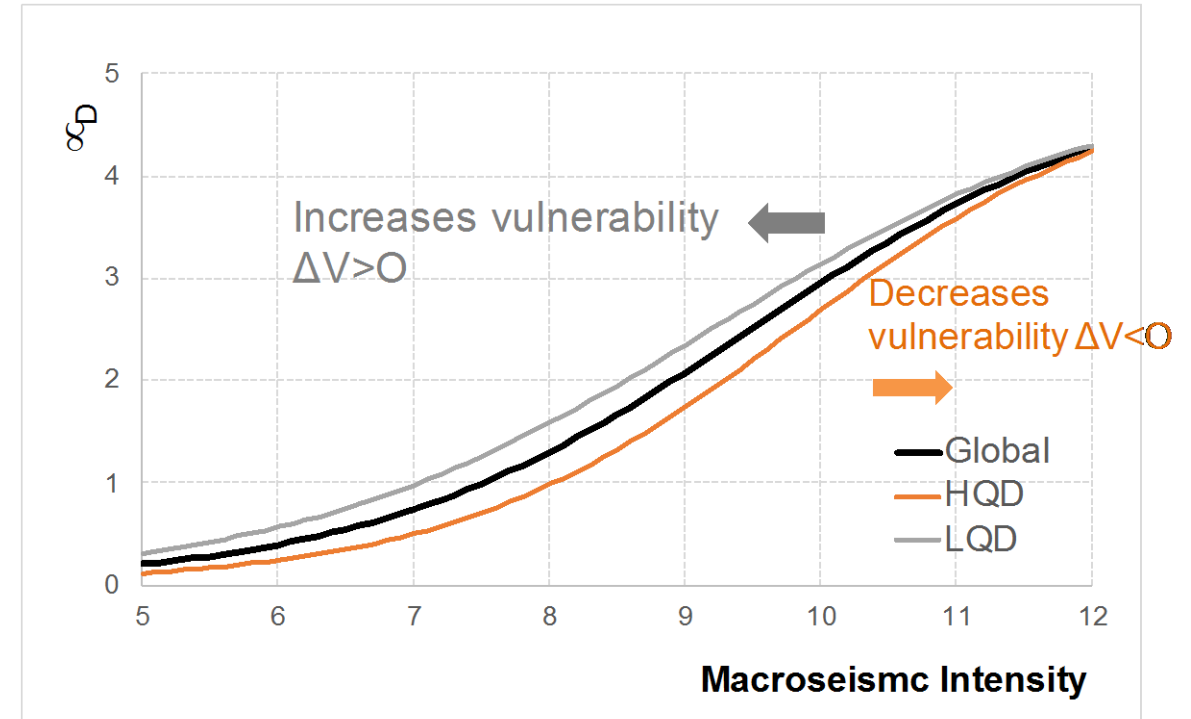
Independent Process



Vulnerability Index

$V_{\text{Global}} \text{ Abruzzo} = 0.692$ [40781] Buildings
 $V_{\text{Regular Cut}} \text{ Abruzzo} = 0.426$ [12369] Buildings
 $V_{\text{Uncut}} \text{ Abruzzo} = 0.814$ [25763] Buildings

ROLE OF MASONRY TYPE



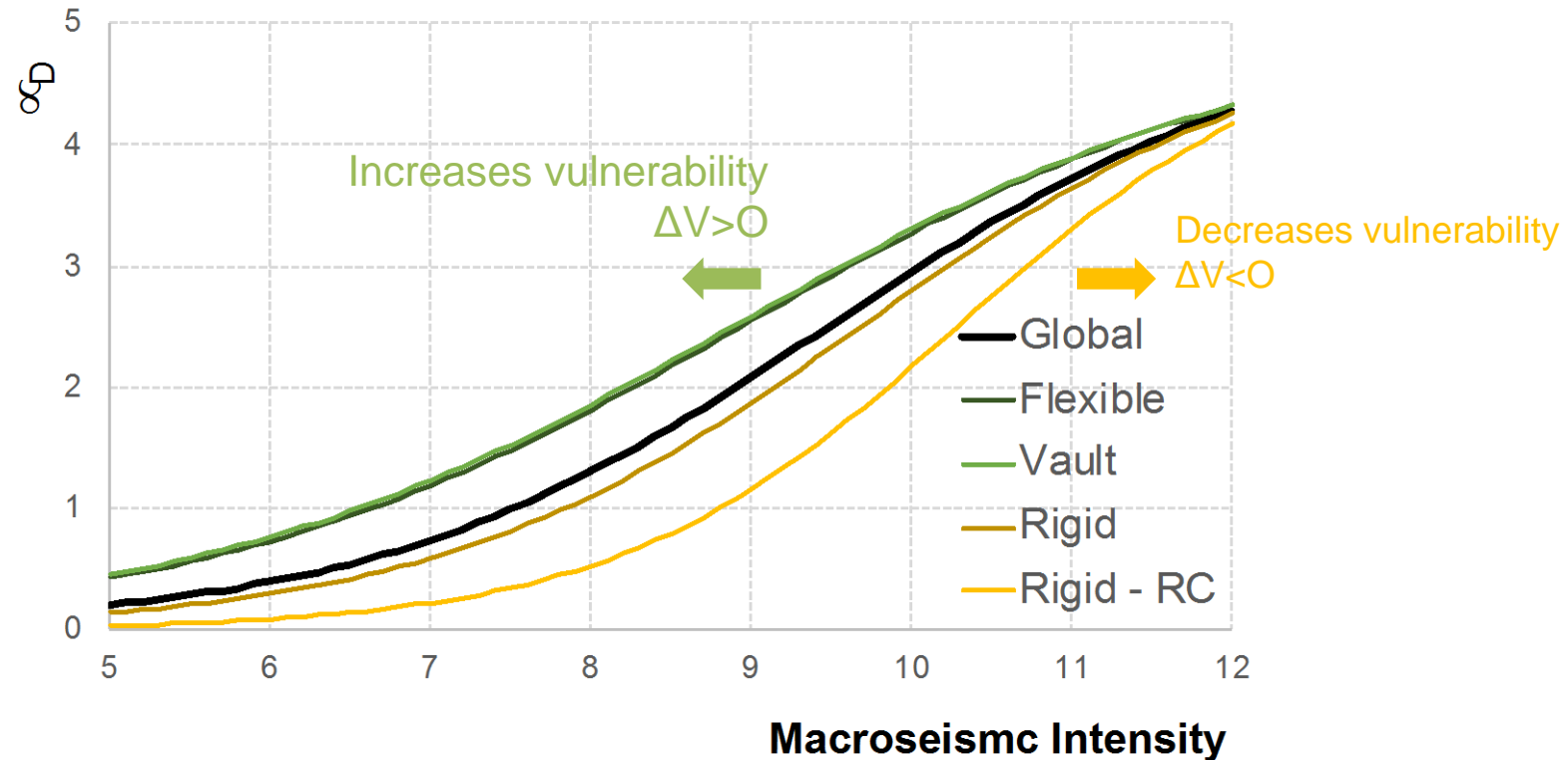
Vulnerability Index

$V_{\text{Global}} \text{ Abruzzo} = 0.692$ [40781] Buildings
 $V_{\text{HQD}} \text{ Abruzzo} = 0.594$ [13749] Buildings
 $V_{\text{LQD}} \text{ Abruzzo} = 0.786$ [24200] Buildings

ROLE OF STRUCTURAL DETAILS

ROLE OF DIAPHRAGMS

Indipendent Process



Vulnerability Index

V_{Global} Abruzzo = 0.692 [40781] Buildings]

V_{vault} = 0.881 [3864] Buildings]

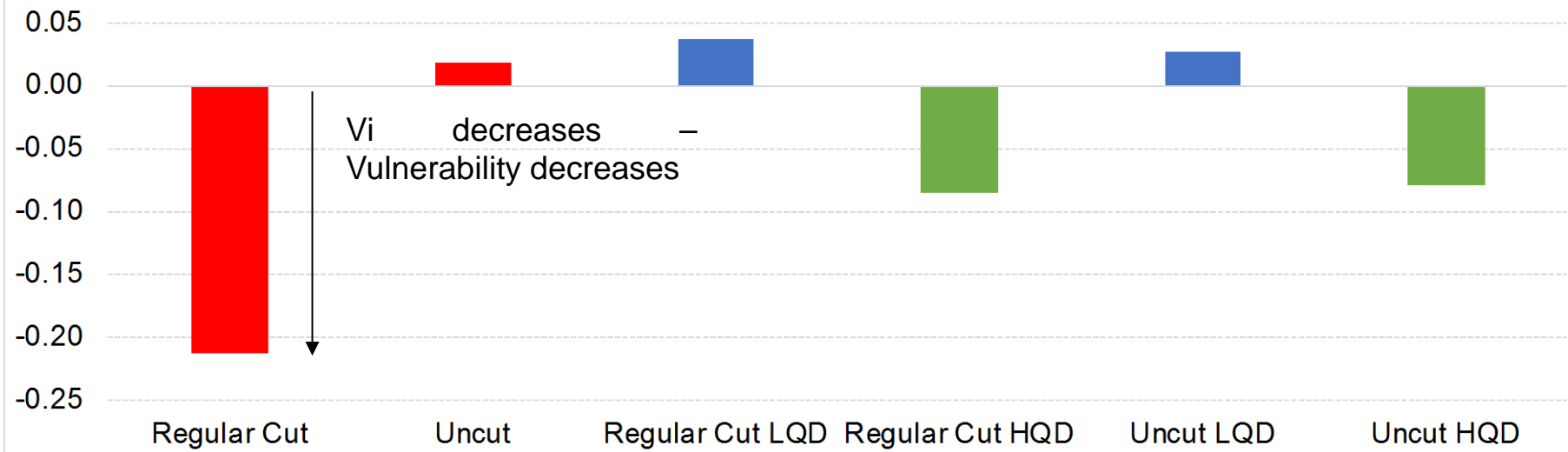
V_{rigid} = 0.865 [5660] Buildings]

V_{rigid} = 0.631 [9442] Buildings]

V_{rigid} = 0.444 [8300] Buildings]

SEISMIC BEHAVIOUR MODIFIERS

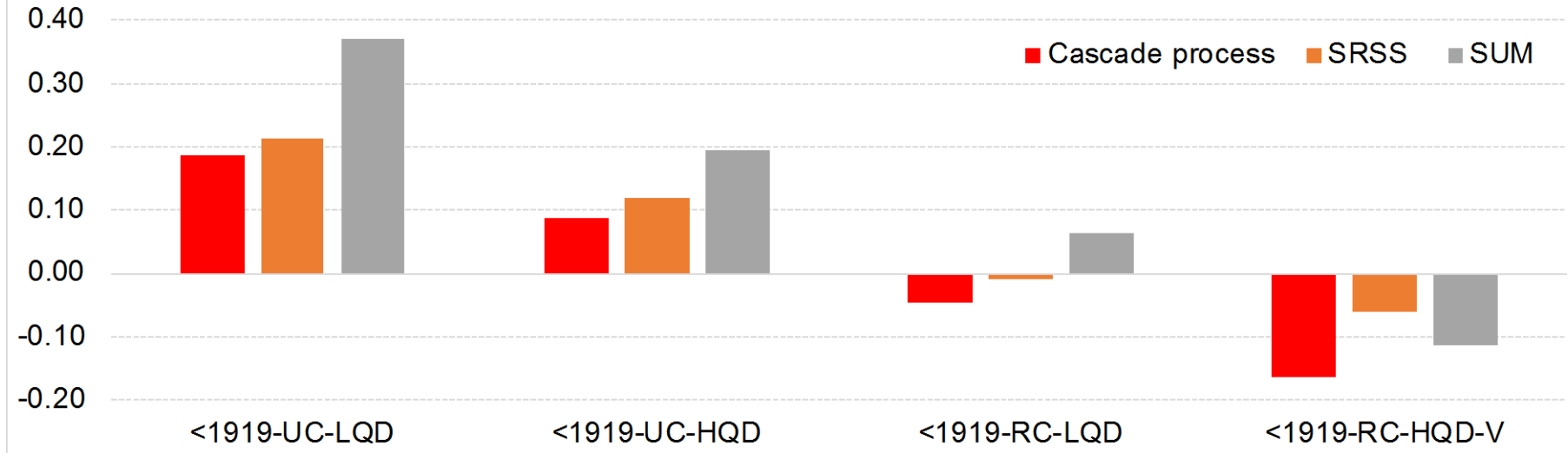
Macroseismic Modifiers - PRE 1919



Modifiers in ΔV_m

- Type of masonry
- LQD: absence of aseismic devices
- HQD: presence of aseismic devices

Macroseismic Modifiers



Modifiers in ΔV_m

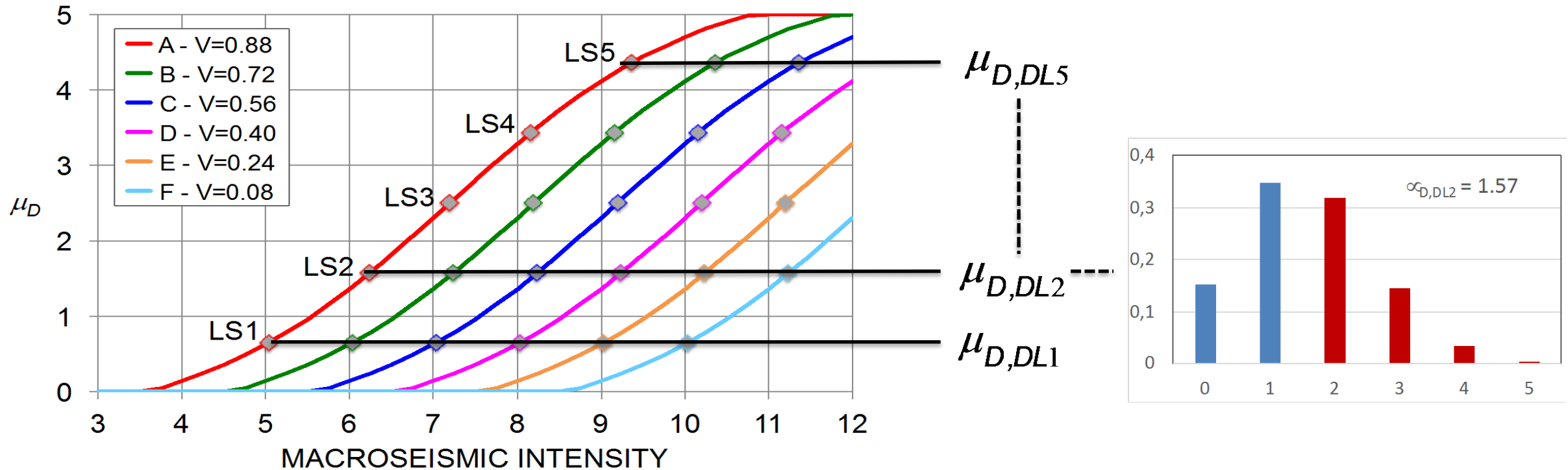
Costruction period < 1919	0.132
Masonry quality - RC	-0.180
Masonry quality - UC	0.127
Detail quality - LQD	0.112
Detail quality - HQD	-0.064

MACROSEISMIC MODEL – from vulnerability curves to fragility curves

□ Firstly, it is necessary to define a reference **MEAN DAMAGE VALUE** to be associated to each **DAMAGE LEVEL**

$$\mu_{D,DLk} = 0.93k - 0.2$$

Linear regression from values obtained from the binomial distribution



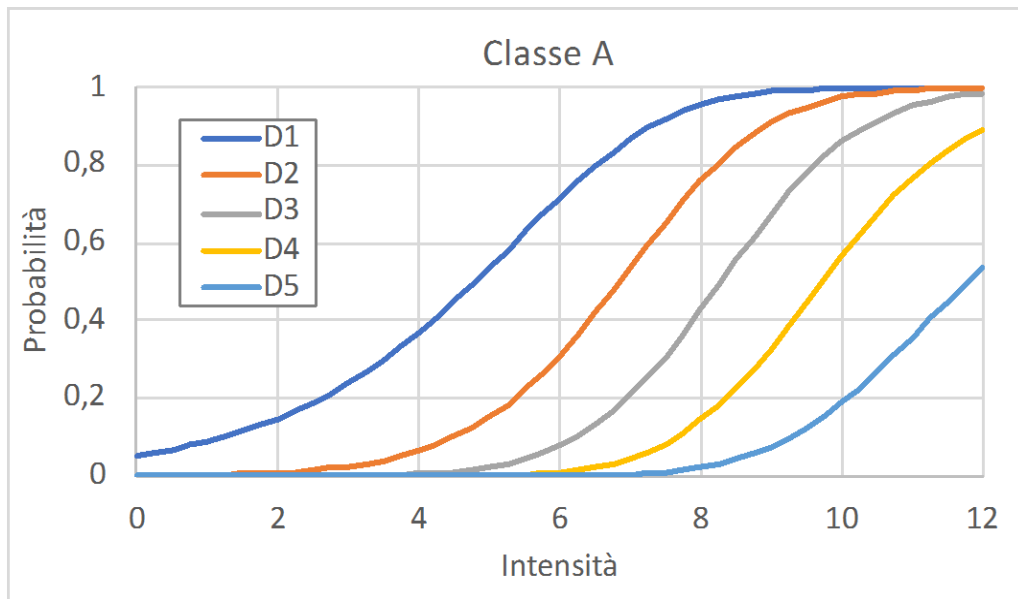
MACROSEISMIC MODEL – from vulnerability curves to fragility curves

- ❑ Firstly, it is necessary to define a reference **MEAND DAMAGE VALUE** to be associated to each **DAMAGE LEVEL**
- ❑ Then, it is possible computing the **fragility curve in terms of Intensity** by assessing the I value that produces the attainment of DL_k

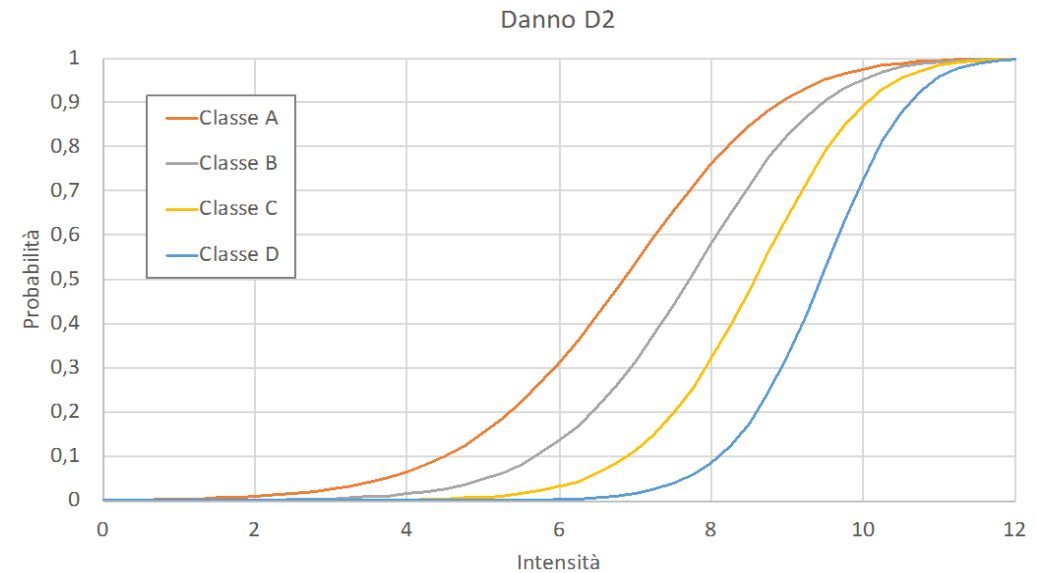
$$I_{DLk} = 11.7 - 3.45V + (0.9 + 2.8V) \operatorname{atanh}(0.4\mu_{D,DLk} - 1)$$

$\mu_{D,DLk} = 0.93k - 0.2$

For a GIVEN vulnerability class varying the DL



For a GIVEN DL varying the vulnerability class

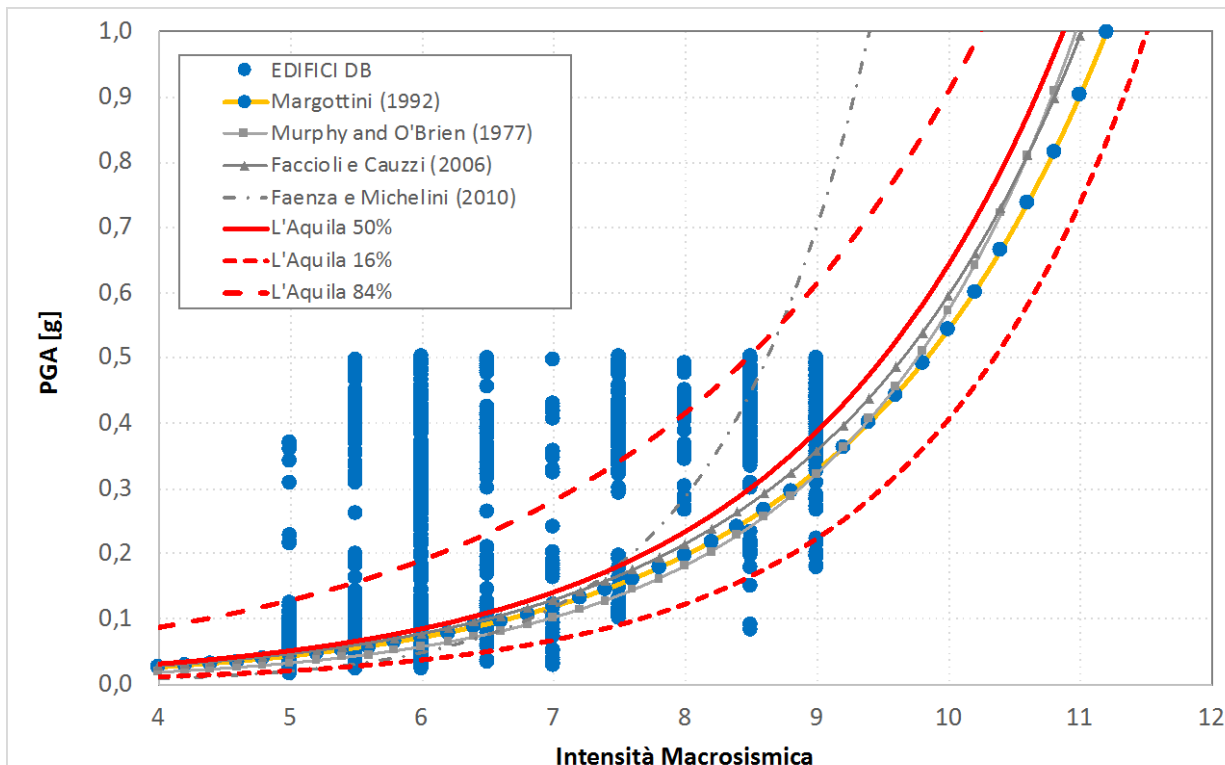


NOT WELL REPRESENTED BY THE CUMULATIVE LOGNORMAL FUNCTION...

MACROSEISMIC MODEL – from vulnerability curves to fragility curves

- ❑ Firstly, it is necessary to define a reference **MEAND DAMAGE VALUE** to be associated to each **DAMAGE LEVEL**
- ❑ Then, it is possible computing the **fragility curve in terms of Intensity** by assessing the generic I value that produces the attainment of DLk
- ❑ Finally, it is necessary to introduce a proper **Intensity – PGA correlation law** in order to define the fragility curve in terms of a instrumental intensity measure

Comparison between some I-PGA Correlation law available in literature and that calibrated by UNIGE on basis of shakemap data from L'Aquila 2009 earthquake



$$\text{Log}(PGA) = a I + b$$

$$I = A \text{Log}(PGA) + B$$

$$PGA = c_1 c_2^{I-5}$$

Literature proposals

Correlazione I-PGA	c_1	c_2
Margottini et al. (1992)	0.0430	1.66
Murphy and O'Brien (1977)	0.0322	1.78
Faccioli e Cauzzi (2006)	0.0464	1.67
Faenza e Michelini (2010)	0.0197	2.44

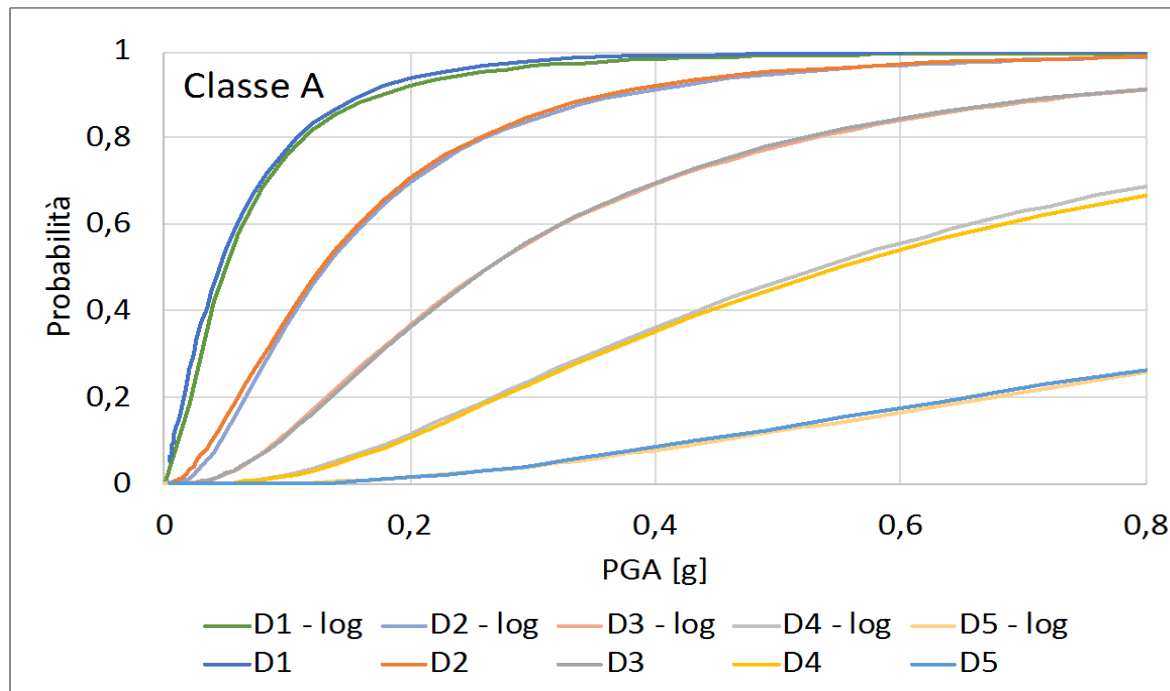
UNIGE proposal

Correlazione I-PGA	c_1	c_2
da ShakeMap L'Aquila (mediana)	0.05	1.66
da ShakeMap L'Aquila (16%)	0.02	1.82
da ShakeMap L'Aquila (84%)	0.13	1.48

MACROSEISMIC MODEL – from vulnerability curves to fragility curves

- ❑ Firstly, it is necessary to define a reference **MEAND DAMAGE VALUE** to be associated to each **DAMAGE LEVEL**
- ❑ Then, it is possible computing the **fragility curve in terms of Intensity** by assessing the generic I value that produces the attainment of DLk
- ❑ Finally, it is necessary to introduce a proper **Intensity – PGA correlation law** in order to define the **fragility curve in terms of a instrumental intensity measure**

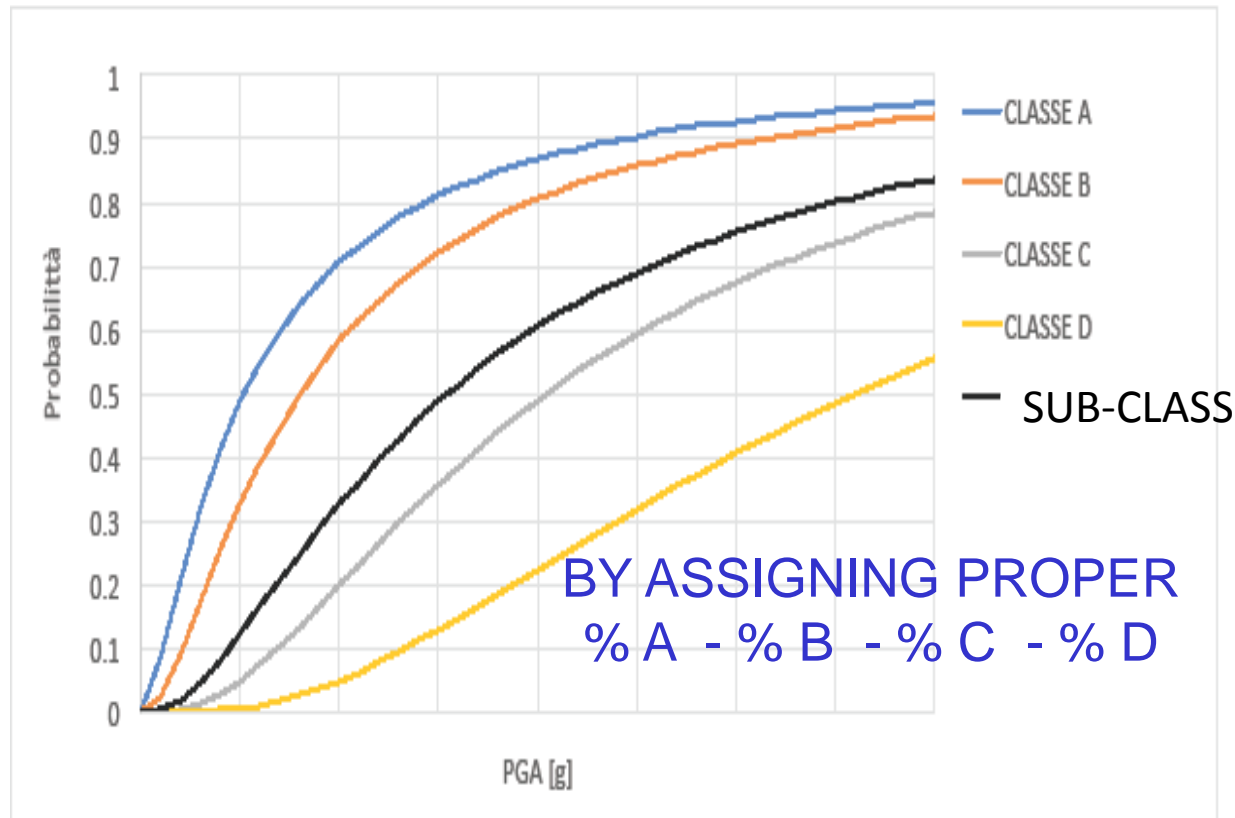
$$PGA_{Dk} = c_1 c_2^{(I_{Dk}-5)} = c_1 c_2^{[6.7-3.25V+(0.9+2.8V)atanh(0.36k-1.08)]}$$



The fragility curve in PGA is well fitted by the lognormal cumulative function

$$p_{LS}(im) = P(d > D_{LS}|im) = P(im_{LS} < im) = \Phi\left(\frac{\log\left(\frac{im}{IM_{LS}}\right)}{\beta_{LS}}\right)$$

HOW WE CAN PASS FROM THE FRAGILITY CURVE OF THE **EMS98 VULNERABILITY CLASSES** TO **OTHER SUB-CLASSES** (→ TARGETED TO OUR INVENTORY & OUR AVAILABLE DATA)?



Class	A	B	C	D
V	0.99	0.80	0.61	0.42

LOW					
Classi età	V _{EMPIRICI}	A	B	C	D
< 1919	0.952	80	20		
1919 - 1945	0.847	25	75		
1946 - 1961	0.705		50	50	
1962 - 1981	0.550			70	30
> 1981	0.420				100

MEDIUM					
Classi età	V _{EMPIRICI}	A	B	C	D
< 1919	0.914	60	40		
1919 - 1945	0.781		90	10	
1946 - 1961	0.743		70	30	
1962 - 1981	0.648		20	80	
> 1981	0.496			40	60

For the aim of validation and within the context of ReLUIS-DPC project addressed to developing Italian seismic risk map the fragility curves have been implemented in the **IRMA Platform**

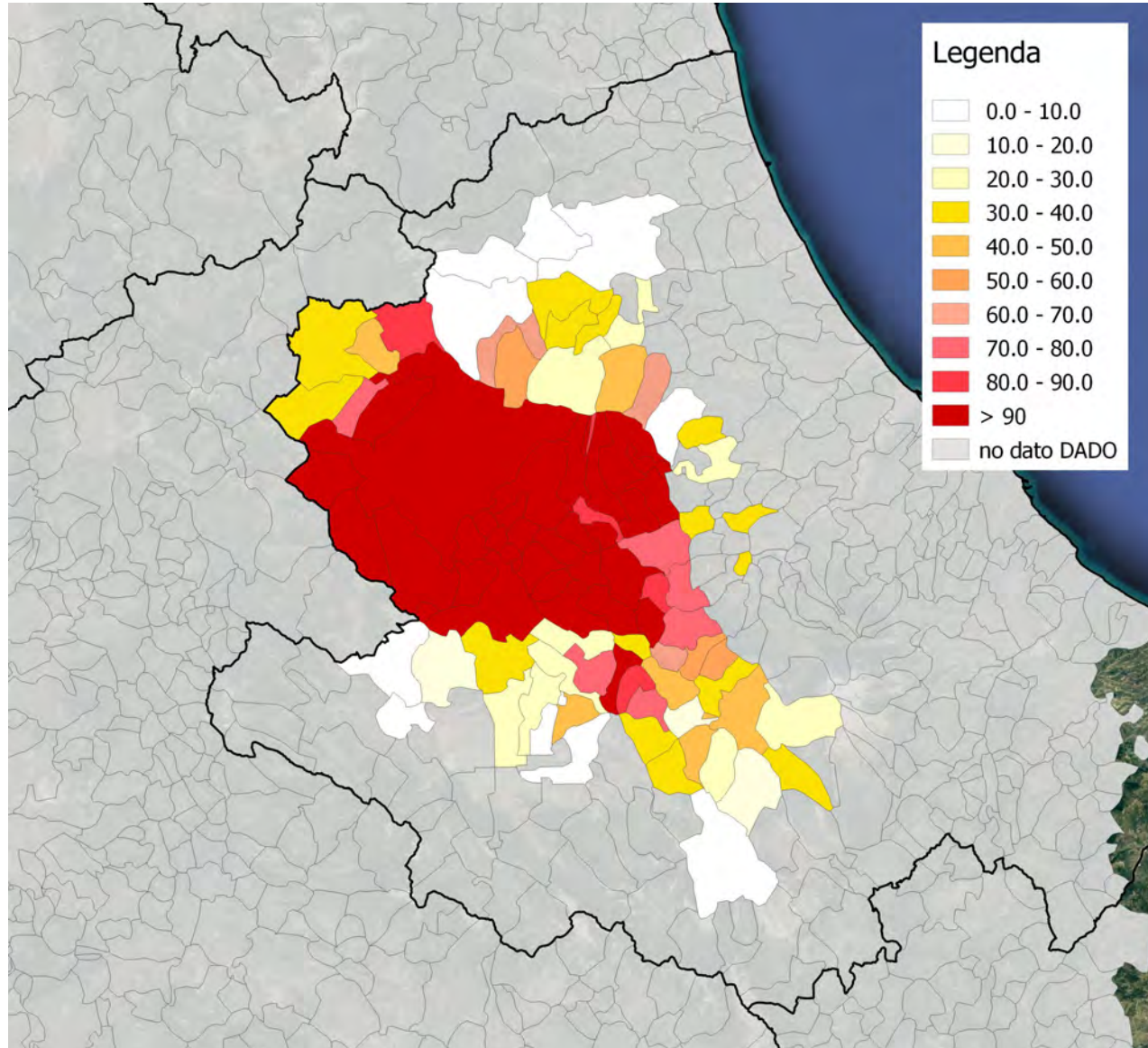
ReLUIS-DPC Project: Italian seismic risk map



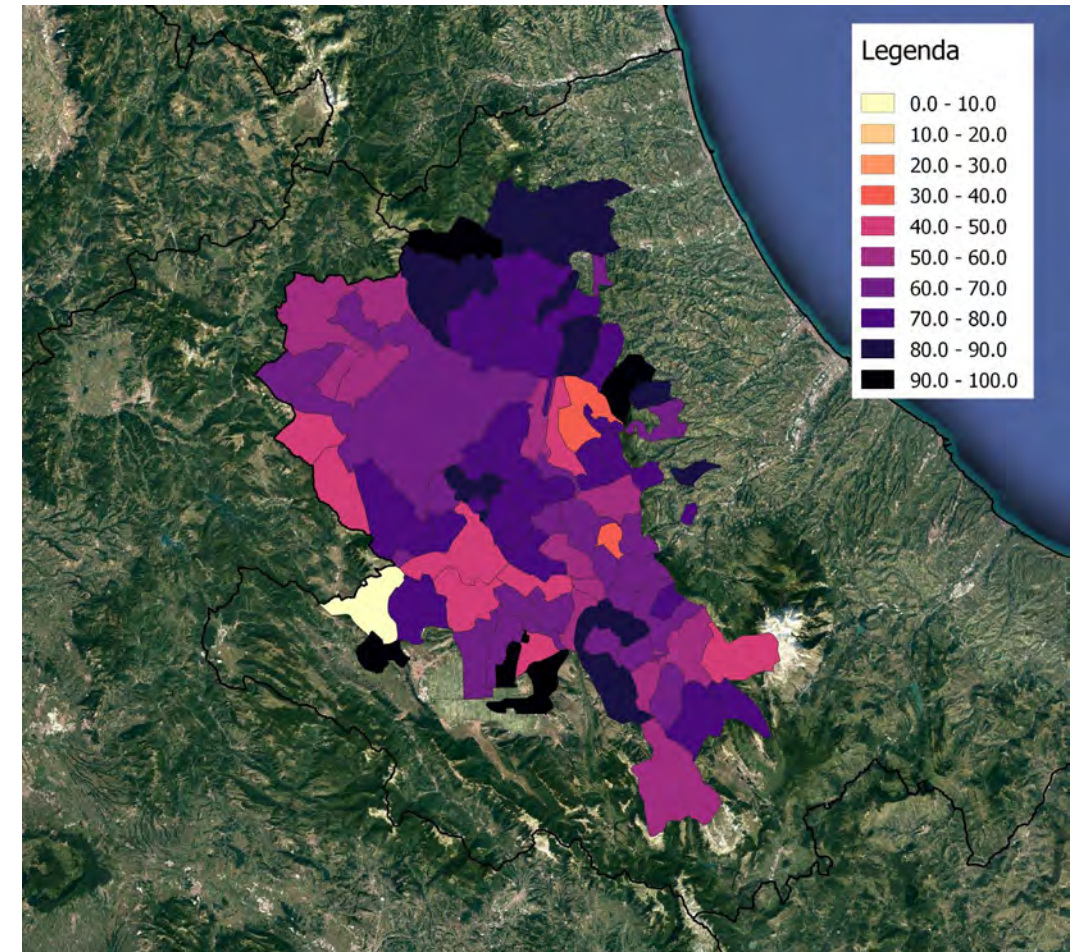
Borzi, B., Faravelli, M., Onida, M., Polli, D., Quaroni, D., Pagano, M., Di Meo, A., 2018. Piattaforma Irma (Italian Risk Maps). *37esimo Convegno Nazionale GNGTS*. 19-21 Novembre 2018, Bologna.

NEW MACROSEISMIC MODEL –VALIDATION

Scenario of L'Aquila 2009 earthquake – *Validation made by the Platform IRMA*



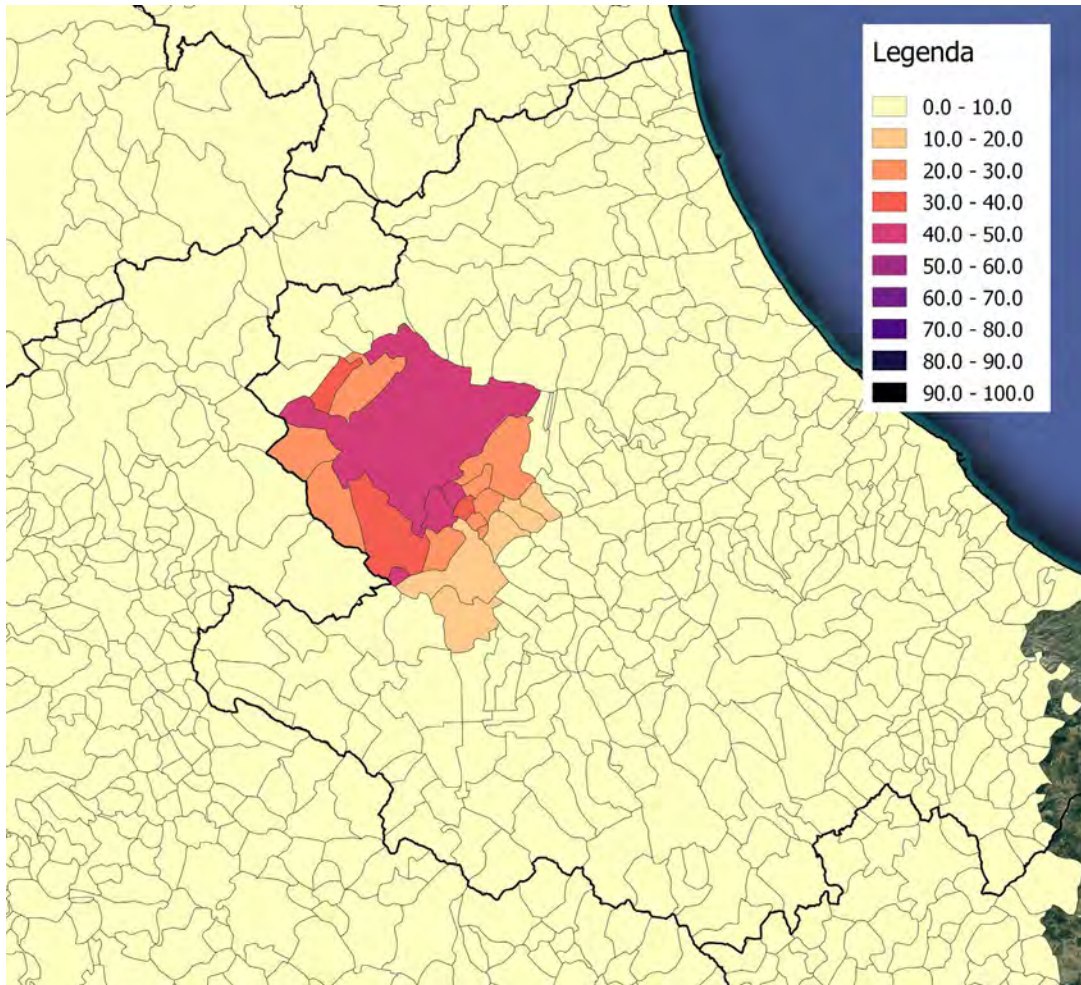
DL1 – real data from
DaDO



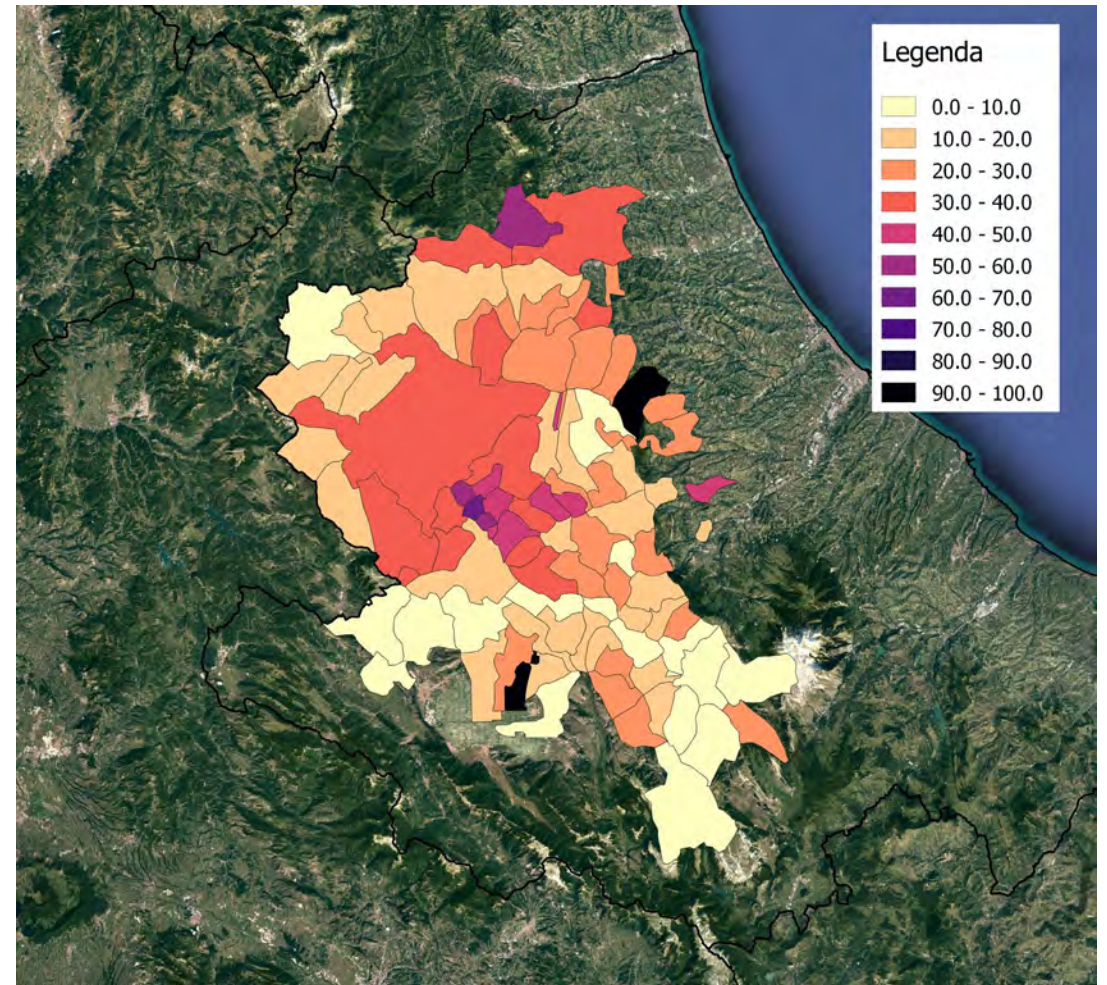
NEW MACROSEISMIC MODEL –VALIDATION

Scenario of L'Aquila 2009 earthquake- *Validation made by the Platform IRMA*

DL3 – Simulated by the
macroseismic model



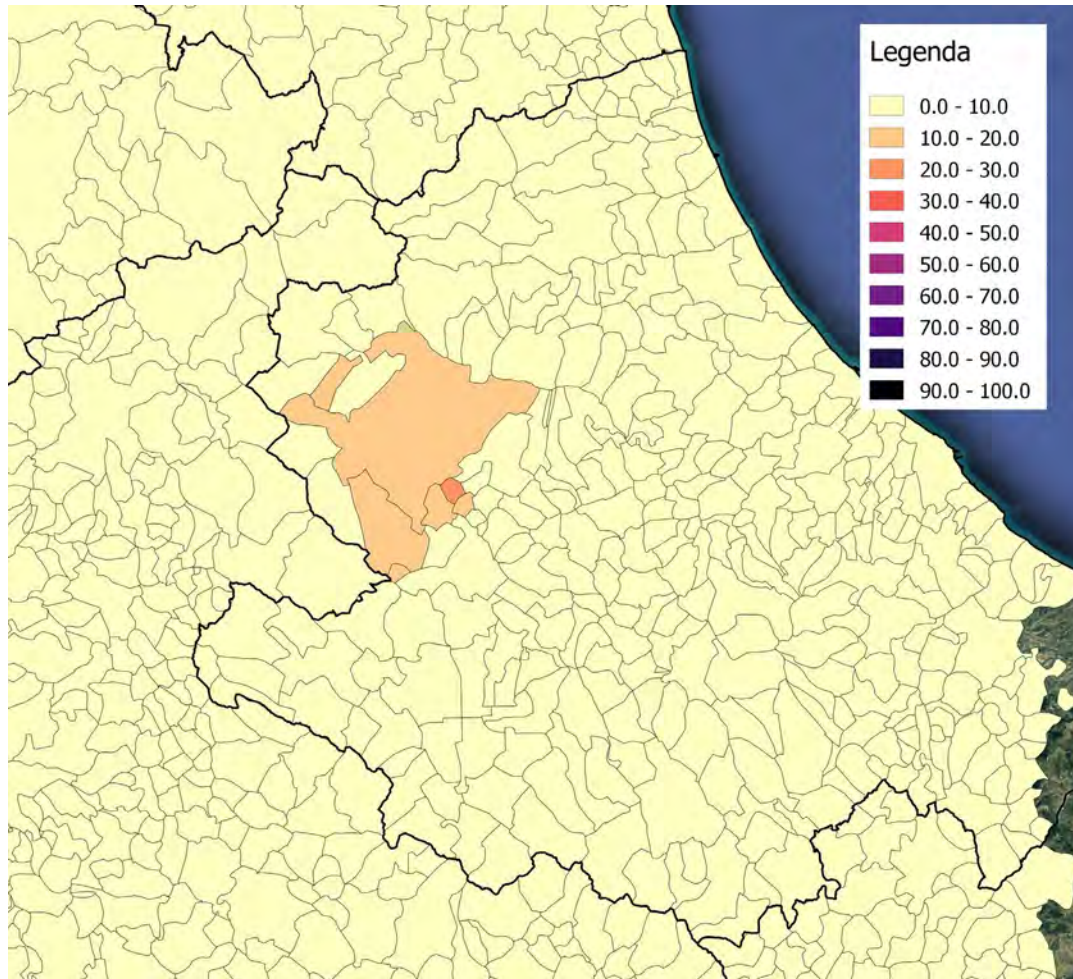
DL3 – real data from
DaDO



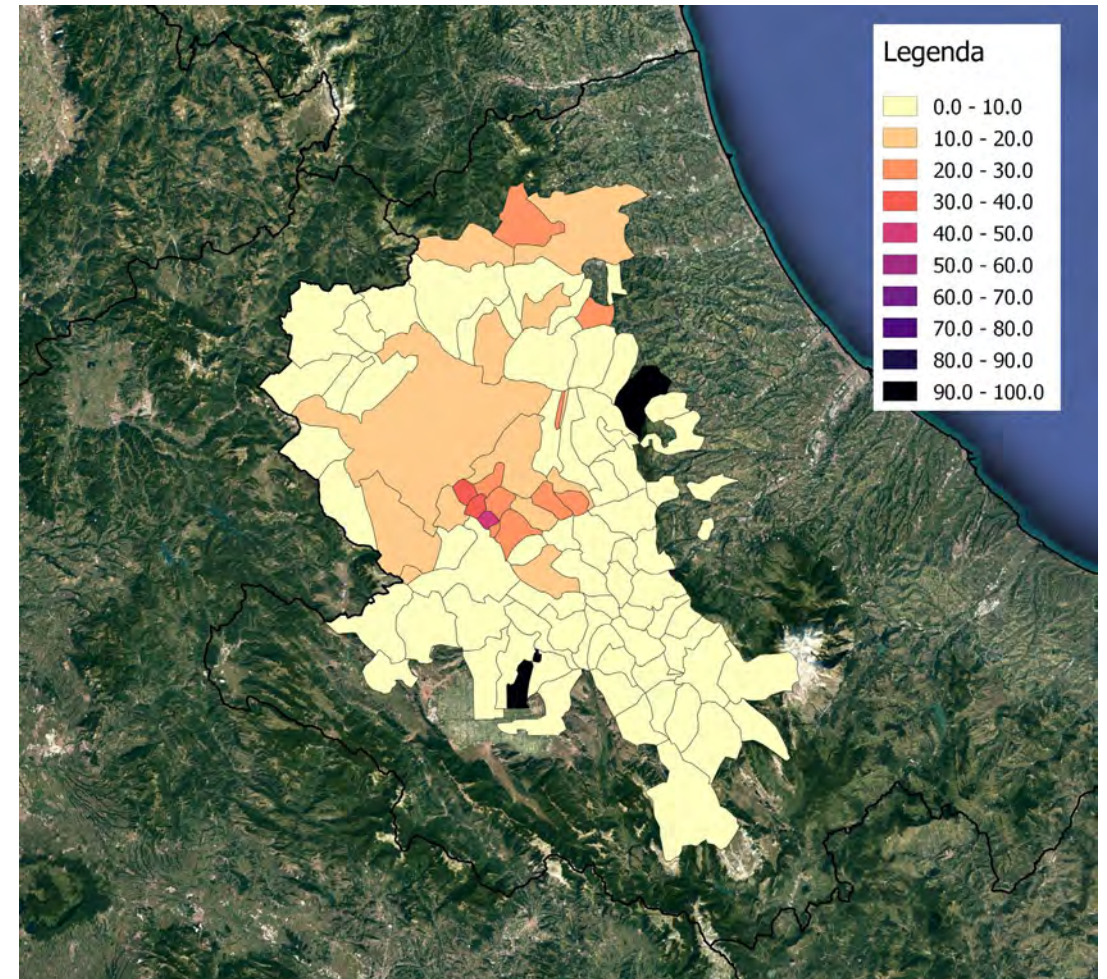
NEW MACROSEISMIC MODEL –VALIDATION

Scenario of L'Aquila 2009 earthquake - *Validation made by the Platform IRMA*

DL4 – Simulated by the
macroseismic model

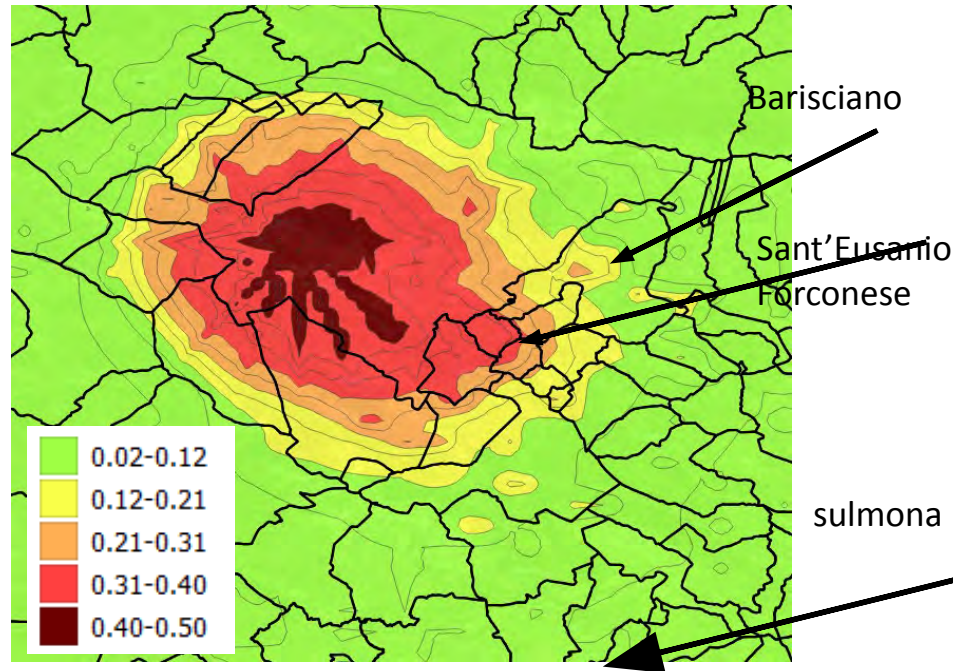


DL4– real data from
DaDO

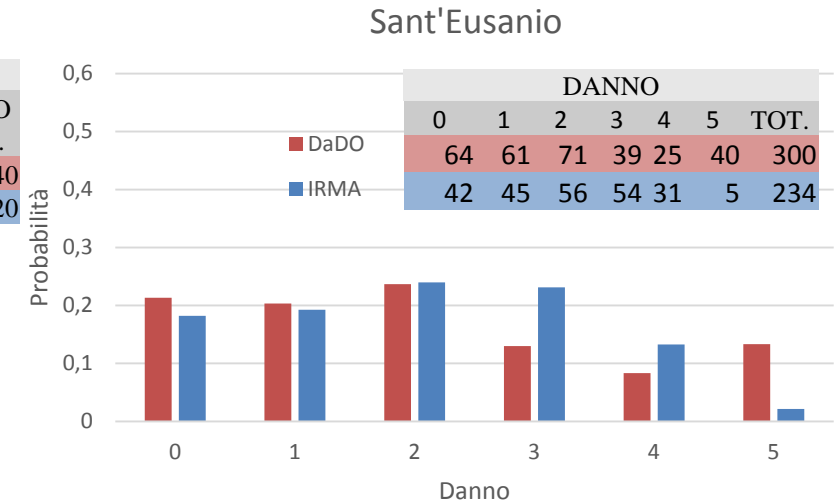
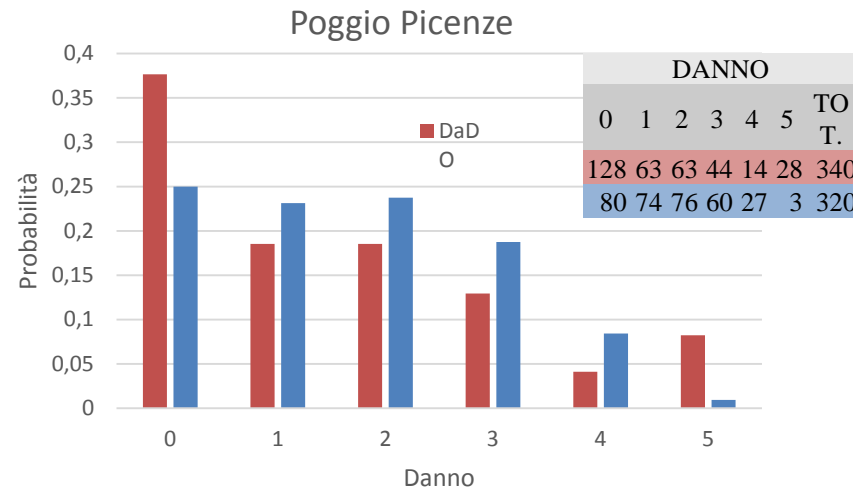
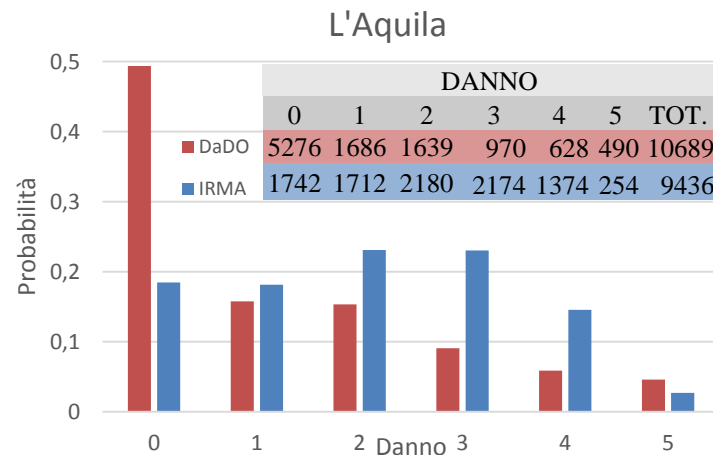
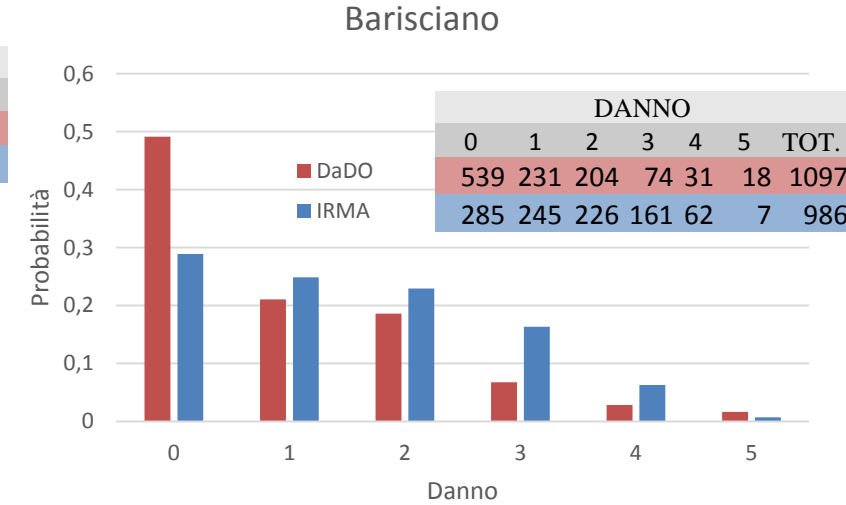
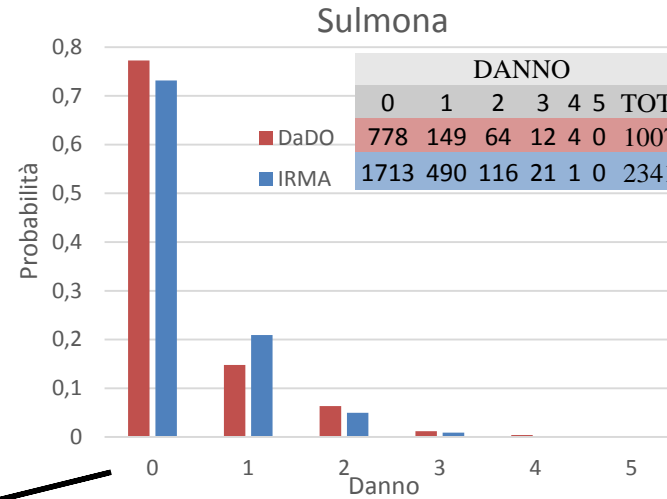


NEW MACROSEISMIC MODEL –VALIDATION

Shakemap of L'Aquila 2009



COMPARISON IN TERMS OF DPM FOR VARIOUS MUNICIPALITIES WITH DIFFERENT EPICENTRAL DISTANCE



OUTLINE OF THE PRESENTATION

FRAGILITY CURVES

What do they represent?

Vulnerability as a component of seismic risk and loss assessment

What do they depend on?

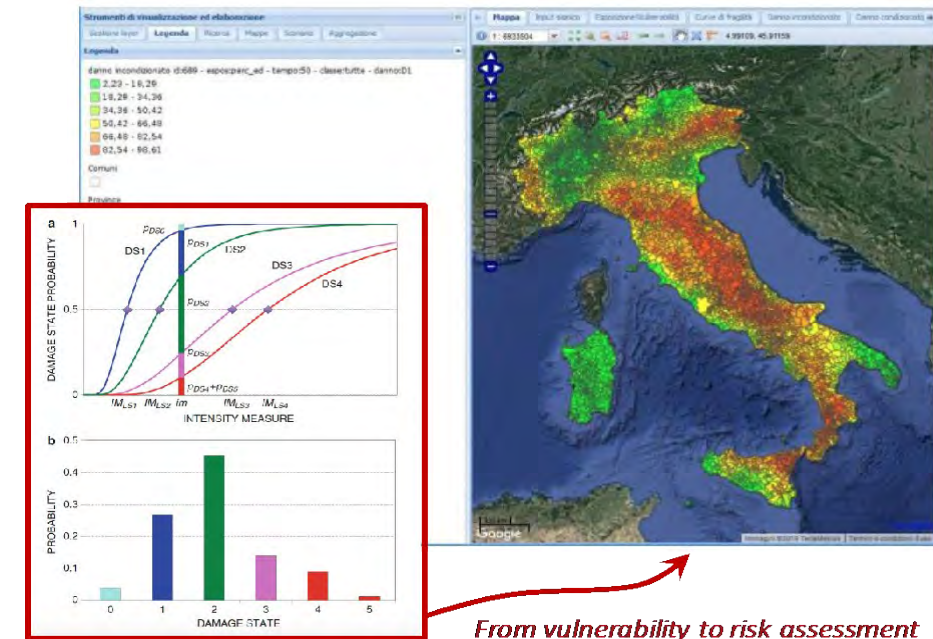
Involved dispersions & influence on results of seismic risk analysis

How are they obtained?

Overview of methods & focus on macroseismic and empirical ones

How can they be used?

Practical issues & application to the Italian seismic risk assessment



IRMA Platform has been used in 2018 for the National Risk Assessment (Italian seismic risk maps and losses) and is going to be improved in the MARS project (Coord. S. Lagomarsino & A. Masi)



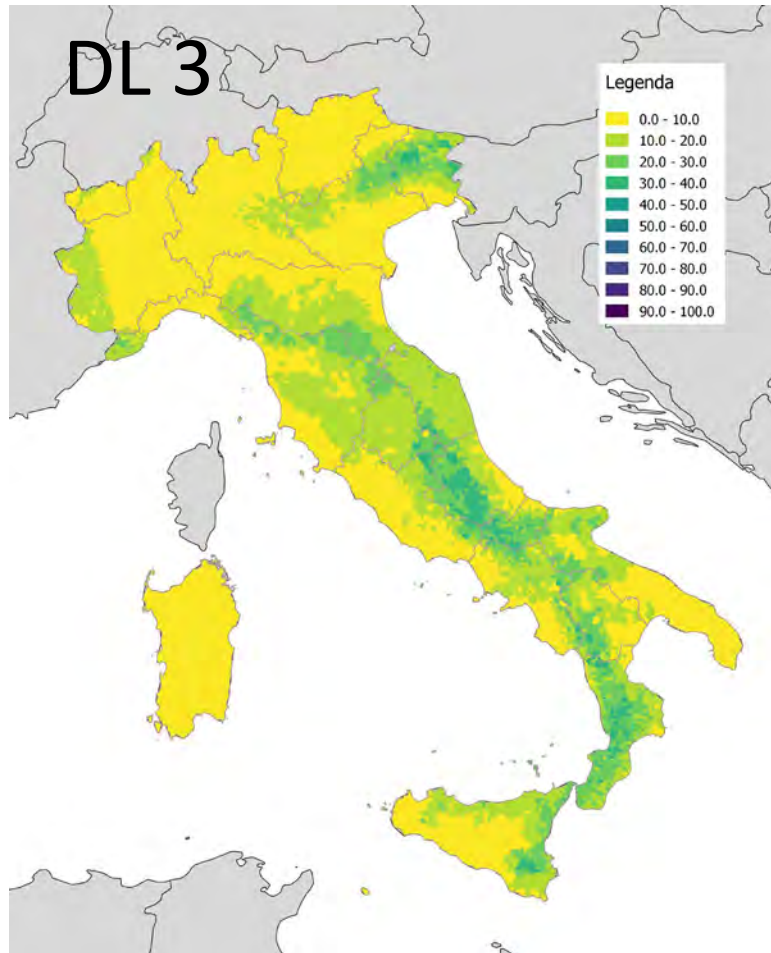
National Risk Assessment (2018) Overview of the potential major disasters in Italy: seismic, volcanic, tsunamis, hydro-geological/hydraulic, extreme weather, droughts and forest fire risks, Presidency of the Council of Ministers Italian Civil Protection Department.

Dolce et al. (2019) Seismic risk maps for the Italian territory, XVIII ANIDIS Conference, Ascoli Piceno 2019

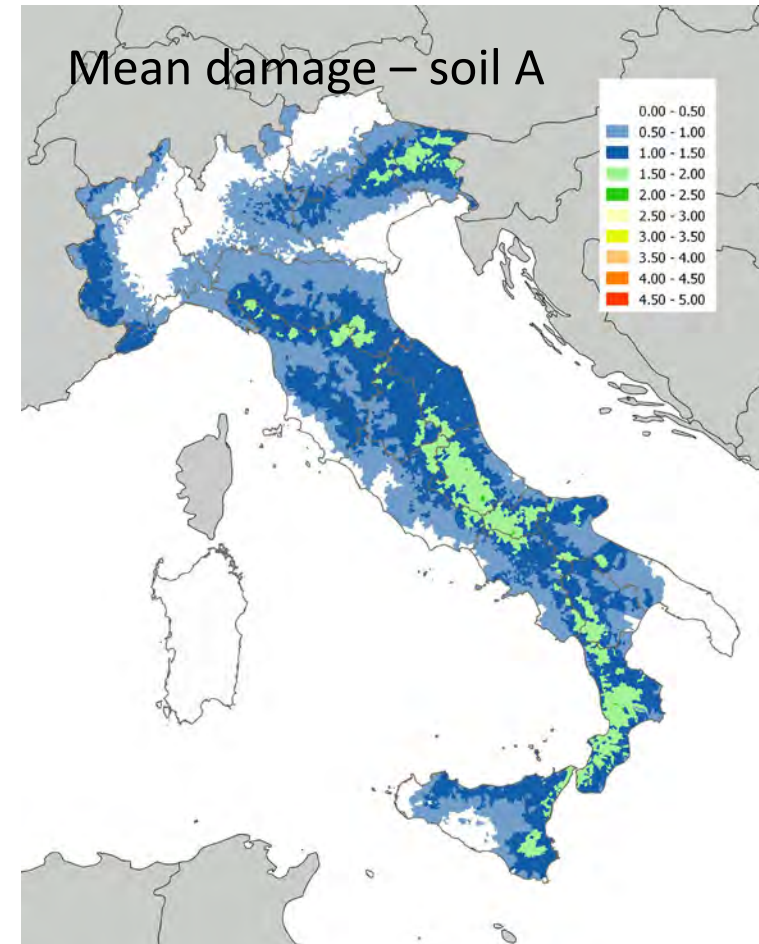
EXAMPLES OF MAPS THAT CAN BE PRODUCED (through the implementation of the macroseismic model developed by UNIGE)

Classification: MATERIAL – AGE – HEIGHT

Scenario conditioned to 475 years – soil A



Scenario unconditioned to 1 year



Other research groups participated to ReLUIS project (from Padua, Naples, Pavia) by defining fragility curves through different approaches (empirical, hybrid mechanical-based)

The result of maps in terms of **damage scenario** have been used to assess also the **expected LOSSES**

↑
It requires the introduction of proper correlation laws
↑

EXAMPLES of correlation laws between the DAMAGE LEVELS and:

CASUALTIES

Loss of life or serious injury

Perdite umane	D4	D5
Vittime	1 %	10 %
Feriti	5 %	30 %



MESSINA 1908

USABILITY

SAFE FOR USE/NOT SAFE FOR USE/COLLAPSE

Livello di danno	D1	D2	D3	D4	D5
Agibili	100	60	0	0	0
Inagibili b.t.	0	40	40	0	0
Inagibili l.t.	0	0	60	100	0
Crolli	0	0	0	0	100



IRPINIA 1980

DIRECT ECONOMIC LOSS RECONSTRUCTION COSTS


CU (€/m ²)	D1	D2	D3	D4	D5
1350	2	10	30	60	100



Other research groups participated to ReLUIIS project (from Padua, Naples, Pavia) by defining fragility curves through different approaches (empirical, hybrid mechanical-based)

RESULT FROM THE UNCONDITIONED EVALUATION AT 1 YEAR

CASUALTIES



	Vittime	Feriti	Senzatetto
Media	505	1.744	78.602
Massimo	763	2.588	131.952
Minimo	123	469	4.0381

DIRECT ECONOMIC LOSS RECONSTRUCTION COSTS

COST in Billions

USABILITY

	Costi	Inagibili b.t.	Inagibili l.t.
Media	2,13	20.938	15.635
Massimo	3,27	31.847	22.024
Minimo	1,27	9.962	7.404

REASONABLE NUMBERS IF COMPARED WITH THE EARTHQUAKE HISTORY OF LAST 50 YEARS IN ITALY BUT **SIGNIFICANT DISPERSION** DUE TO DIFFERENCES IN VARIOUS MODELS ADOPTED

RESEARCH ONGOING IN 2019 WITHIN MARS – ReLUIIS PROJECT

THANK YOU FOR YOUR
KIND ATTENTION

Sergio Lagomarsino
sergio.lagomarsino@unige.it



**Università
di Genova**

**DICCA – Department of Civil,
Chemical and Environmental
Engineering**